



November 30, 2023
File: LD-00318

VIA EMAIL

The Municipality of Middlesex Centre
1 Tunks Lane
Komoka, ON N0L 1R0

Attention: Mr. Robert Cascaden, Director of Public Works and Engineering

Reference: MN 10919 Longwoods Road, Delaware
10919 Longwoods Road Inc.
Stormwater Servicing Brief

Introduction

10919 Longwoods Road Inc. has retained LDS Consultants Inc. (LDS) to prepare a stormwater management (SWM) strategy in support of a proposed draft plan redline for a vacant land condominium development in the community of Delaware, as illustrated in **Figure 1**. The subject site comprises agricultural lands currently used as a meadow measuring approximately 6.65 hectares. The site is bound to the north by Longwoods Road, to the south and west by agricultural lands and to the east by low-density residential properties fronting Carriage Road.

Background Information

The proposed stormwater management strategy was developed using the following information presented in the following reports:

- Middlesex Centre Infrastructure Design Standards, dated January 2018;
- Stormwater Management Policy Manual of the Municipality of Middlesex Centre, prepared by Stantec Consulting Ltd., dated June 2011;
- Stormwater Management Planning and Design Manual, organized by the Ministry of the Environment, dated March 2003; and,
- Geotechnical Investigation & Preliminary Hydrogeological Assessment, prepared by LDS Consultants Inc., dated November 2023;

Design Criteria

The SWM outlet for the site is located within the Lower Thames River Watershed. Accordingly, the following SWM design criteria apply to the proposed development area:

- *Water Quantity Control* - Ensure that stormwater flows are attenuated to pre-existing rates on-site, up to the 100-year event. Outflows from the SWM facility (SWMF) will need to be restricted to the capacity of the pipe of the proposed municipal drain for up to the 25-year design storm event. Major flows must be safely conveyed via an Overland Flow Route (OLFR) to an appropriate outlet without causing damage to private property or municipal infrastructure and with minimum risk to the public.
- *Water Quality Control* - Provide quality control based on 'Enhanced' Level 1 treatment (MOE, 2003).

Existing Condition

Google Maps© shows that the site comprises agricultural lands used as a meadow. Based on the watershed sketch by Spriet Associates London Ltd., a portion of the neighbouring properties and Longwoods Road drain towards the low-lying area of the subject site. Subsequently, stormwater infiltrates the ground where a subsurface agricultural drainage system drains the water south towards a natural surface water ravine feature adjacent to the Elviage Drain, which ultimately discharges into the Thames River. Existing catchment areas are described below. The site was modelled using the

SWMHYMO hydrologic software to calculate existing peak flows summarized in **Table 1**, whereas catchment areas are illustrated in **Figure 2**. Model documentation is attached to this letter report in **Appendix B**.

Catchment 101 – This area comprises the subject property currently used for agriculture and is covered with a meadow. This area drains to a low spot visible with a review of the contours on site, and the southern portion drains to the neighbouring agricultural field.

External Catchment EX-1 – This area comprises a portion of the neighbouring agricultural field located west of the subject site, which flows towards the low spot in *Catchment 101*.

External Catchment EX-2 – This area comprises a neighbouring single-family residential property located northwest of the subject site, which flows towards the low spot in *Catchment 101*.

External Catchment EX-3 – This area comprises half the Longwoods Road right-of-way (ROW) fronting the subject property. Stormwater runoff flows towards the low spot in *Catchment 101*.

External Catchment EX-4 – This area comprises a portion of the neighbouring agricultural field located east of the subject site and a portion of the low-density residential lotting fronting Carriage Road. Stormwater runoff flows towards the low spot in *Catchment 101*.

Table 1 - Existing Runoff Peak Flows

Storm Event	Surface Runoff (m ³ /s)
2-year	0.089
5-year	0.194
10-year	0.266
25-year	0.345
50-year	0.410
100-year	0.474
250-year	0.700

LDS conducted a geotechnical investigation to determine existing soil conditions at the subject site. Generally, soils observed in the boreholes consisted of topsoil overlying natural sand and silt soils. Therefore, LDS classified the soils within the Hydrologic Soil Group B category for modelling purposes.

Proposed Condition

It is proposed to develop the site into a light industrial type development comprising eighteen (18) vacant land condominium lots. The proposed development will include paved areas and some grass-landscaped areas. The internal storm sewer will convey runoff generated by minor storm events from the external area and on-site to the proposed municipal drain. Similarly, the OLFR will convey major event-generated runoff to the dry pond, which flows to the proposed municipal drain via the orifice-restricted outlet and emergency overflow structure. The proposed catchments are described below and illustrated in **Figure 3**. External drainage catchment areas will remain as described in this report's existing conditions section. The proposed condition was modelled using SWMHYMO software to estimate post-development peak flow rates, which are included in **Appendix B**.

Catchment 201 – This catchment area comprises the developed site. This area contains the 18 proposed industrial lots, the internal street network with grassed landscaped areas and SWMF. In addition, the sewer system, dry pond and site grading will convey minor and major flows to the site's southwest corner. This catchment will accept overland flows from *Catchment EX-1*.

Quantity Control

Under proposed conditions, an on-site detention strategy controls additional runoff generated by the development to meet water quantity objectives established by the MECP (Ministry of Environment, Conservation and Parks), Spriet Associates London Ltd. and the Municipality of Middlesex Centre. This strategy comprises a combination approach of an on-site storm sewer network, a dry pond, and an orifice-controlled outlet. It is proposed to provide forty-three hundred (4300) cubic metres of storage in surface ponding and underground storm sewers and maintenance holes, with the majority contained in the dry pond for peak flow attenuation. During major storm events, ponding will occur on the

private roadway. The SWM strategy will attenuate post-development stormwater runoff flow rates for all storm events, including the 250-year storm within the storage elements described above and released through an orifice-controlled pipe outlet and emergency overflow structure connected to the proposed municipal drain. The major storm event hydrographs are included in **Appendix B**. The stage-storage-discharge relationship for the 135 mm orifice-controlled system is outlined in **Table 2**. Orifice sizing is based on the capacity constraints of the outlet drain designed by Spriet Associates London Ltd.

Table 2 - Stage-Storage-Discharge for Orifice-Controlled Outlet

Stage	Total Volume	Discharge	Comment
(m)	(m ³)	(m ³ /s)	
232.83	0	0.000	Orifice Invert
236.20	188	0.059	Bottom of Dry Pond
236.50	2700	0.087	Top of Dry Pond
236.65	4003	0.299	Site Emergency Overland Spill Point

The resulting peak flows are presented in **Table 3**.

Table 3 – Post-Development Runoff Peak Flows

Storm Event	Municipal Drain (m ³ /s)	Infiltration (m ³ /s)
2-year	0.069	0.009
5-year	0.073	0.010
10-year	0.075	0.010
25-year	0.101	0.011
50-year	0.137	0.011
100-year	0.170	0.011
250-year	0.286	0.011

All supporting calculations are included in **Appendix C**.

Quality Control

The MECP recommends that an Enhanced Level of Protection be provided. Quality control will be achieved with the use of OGS technology. The OGS device will be placed at the site’s storm sewer outlet and treats up to the 50-year storm event. The OGS will be sized to achieve above 80% total suspended solids (TSS) removal. This reaches the Enhanced Level of Protection recommended by the MECP. The OGS device proposed is the ADS Model FD-8HC or approved equivalent. The OGS device will treat runoff from the entire internal road network and driveways. OGS grit loading information is attached. It is expected that the OGS unit will have to be serviced once every 20-24 months. OGS sizing calculations are included in **Appendix D**.

Operation and Maintenance

During the construction of the SWM facility, it is recommended that monitoring and inspection of the erosion and sediment controls be conducted to ensure the satisfactory performance of these measures.

Reporting of the inspection and monitoring results will be distributed to the Owner. Suppose it is found that the erosion and sediment control measures need to be fixed. Then, based on field decisions, they shall be augmented to the Owner’s satisfaction.

Furthermore, it is recommended that the owner initiate a post-construction monitoring program to ensure the long-term effectiveness of the SWM facility. The post-construction monitoring program should include the following:

- Periodic inspection of the SWM control facility and other erosion control works.
- Inspect the SWM facility and its outlet after significant rainfall events (generally more than 10 mm of rainfall).
- Removal of debris that may accumulate and hinder the functioning of the SWM facility.
- Implement remedial measures, including erosion stabilization, repair of damaged vegetation and sediment removal, as required.

The frequency of the post-construction monitoring will be at the Owner’s discretion. However, it is recommended that a minimum of four (seasonal) inspections be made annually.

Erosion and Sediment Control

This section describes the Erosion and Sediment Control Plan implemented before, during and immediately after construction to reduce the possibility of sediment being conveyed from the proposed construction site.

Types of Selected Erosion/Sediment Control Methods

The details and locations of the proposed temporary and longer-term erosion and sediment control measures will be identified at the detailed design stage. The construction drawings, once complete, will form a part of the sediment and erosion control plan. Proposed erosion and sediment control measures include the following:

- The Contractor will install a light-duty silt fence along the site’s perimeter.
- All disturbed areas where work will not occur for 30 days or more will be stabilized following OPSS 572.
- The Contractor will perform street sweeping to remove soil deposited on adjacent right-of-way by construction traffic.

The proposed temporary erosion and sediment control measures have been selected based on the site’s susceptibility to erosion, the sensitivity of the downstream environment, site slopes, and the total drainage area. The proposed measures should provide adequate erosion and sediment control for the project without additional steps. However, the site will be monitored during construction; other actions will be added if required.

Installation of Erosion Control Measures

Proposed erosion and sediment control measures are summarized in **Table 4**.

Table 4 - Erosion and Sediment Control Sequencing

Erosion and Sediment Control Measures	
Pre-Construction	Create a contact list for emergency contingency plan operations.
	As appropriate, install a silt fence around the proposed work limits and the stormwater facility.
	Install robust siltation barrier.
	Preparation of a Construction Dewatering Discharge Plan, including discharge location and temporary storage locations and identifying measures to reduce suspended solids or other treatment, if required.
Construction	Monitor water quality (turbidity) for construction dewatering discharge water discharged at the surface.
	Regularly inspect erosion and sediment control measures to confirm they are practical and operating as intended.
	Monitor weather reports for significant precipitation events for contingency planning.
	Install filter cloth in on-site catch basins.
	Perform street sweeping as necessary to remove accumulated sediment from the adjacent right-of-way.
	Complete final paving.
Complete final landscaping and vegetation plantings.	
Post-Construction	Remove robust siltation barrier.
	Remove the silt fence from the proposed work limits.
	Remove filter cloth from on-site catch basins.
	Remove the construction fence from the proposed work limits.

The proposed erosion and sediment control measures have been designed according to the site slopes, drainage area, and the risks and consequences of failure. Based on these factors, additional steps will likely be optional.

However, the site will be monitored during construction, and additional measures (i.e., additional rows of silt fence) may be installed at the discretion of the Contract Administrator. Although this is not an exhaustive list, inspections should include checks on siltation barriers to confirm it is properly installed and secured, including a review for evidence of damage, tears, overtopping or undermining; checking the condition of surface water ponding areas and storm drain inlets; and documenting areas where seeding/sodding/mulching is implemented to re-establish vegetative cover. In addition, due to the high groundwater condition, inspections shall include checking exposed banks for seepage. If seepage is encountered, erosion control blankets shall be installed to mitigate any damage to the banks.

The triggers for installing enhanced erosion and sediment control measures would include breaching the proposed erosion and sediment control measures and re-evaluation based on site conditions during construction. As described below, site conditions and the proposed erosion and sediment control measures will be monitored regularly.

Inspection Requirements

The effectiveness of the erosion and sediment control measures will need to be monitored during site grading and site servicing work. This will require frequent inspections. Therefore, the following minimum inspection intervals are recommended:

- The Contractor and Contract Administrator shall monitor weather reports daily and record temperatures and rainfall. When rainfall is anticipated, the Contractor and the Contract Administrator shall inspect the erosion control works immediately before and immediately after the rainfall event and snowmelt event (timing for inspections is based on predicted weather forecasts);
- Daily during extended or significant precipitation (i.e., rainfall amounts that exceed 25 millimetres) or during significant snowmelt periods;
- Daily during any construction activity that would potentially yield significant runoff volumes or otherwise impact the quality of the runoff leaving the site;
- Daily while deficiencies are present which fail to contain, filter or otherwise treat runoff or contribute to sediment loading in surface water;
- Weekly during dry periods while construction activity is occurring at the site. The Contractor and the Contract Administrator shall inspect the erosion control measures the day before the last business day of the week (typically Thursday) to allow any work to be completed on damaged erosion control works before the weekend.; and,
- Monthly during inactive periods (> 30 days).

The Contract Administrator will document all inspection activities in weekly erosion and sediment control reports.

The Contractor shall construct and maintain all erosion and sediment control measures. This shall include but not be limited to preserving fencing and removing accumulated sediment. Temporary erosion and sediment control measures will not be removed until the areas they serve are restored and stable. The builder will remove the erosion and sediment control measures after the sod has been rooted on the site.

Contingency Plan

The contingency plan aims to help minimize the risk or consequence of a failure of the erosion and sediment control works. Failure could result from insufficient measures, lack of maintenance, or severe weather conditions. The contingency plan includes two areas of consideration:

- Procedures that will be followed where a failure has occurred; and
- Contingency measures will be implemented where there is potential for loss.

The Contractor shall be responsible for following the contingency plan and will prepare the following items:

- The Contractor will maintain a contact list for emergencies.
- Workers shall be on call for emergencies for all aspects of the emergency, from design to installation of sediment and erosion control measures. Any associated health and safety issues are the responsibility of the Contractor.
- Sediment and erosion control measures such as erosion control blanket, straw bales and stakes, sandbags, and silt fences shall be available for emergency installation.

- Gas-powered pumps, appropriately sized hoses, filtration hose socks, and filter cloth will be available for emergency dewatering.
- Heavy equipment shall be on standby for emergency works.
- A supplemental contact list for any required equipment or materials shall be prepared and available for emergencies.

Monitoring

As noted previously, regular monitoring of the site's erosion and sediment control measures makes up an integral part of the contingency plan by providing an early indication should environmental control measures (such as sediment and erosion control measures) or practices fail to achieve prescribed standards. Recommended inspection intervals were discussed previously.

If monitoring identifies a high potential for failure, steps will be taken to reduce the risk. These measures may include repairing existing efforts, modifying current measures, and adding new measures.

If unforeseen events cause the strategies set out in the contingency plan to be insufficient or inappropriate to meet the objective of containing sediment within the working limits, the Contractor, either independently or as directed by the Contract Administrator, will respond promptly with all reasonable measures to prevent, counteract, or remedy any effects on aquatic habitat, and human interest (i.e., public safety, property value).

Updates to the Erosion and Sediment Control Plan may be required to reflect changes at the site during various stages of construction. The municipality will be circulated with updated plans to ensure they have the most current techniques available for review and consultation.

Severe Weather Anticipated

In cases where the weather forecast indicates that significant rainfall is expected within 24 hours, the Contractor shall immediately complete the following:

- Inspect existing erosion and sediment control measures to confirm that they are secure and in good working order;
- Review site conditions to identify and protect areas of exposed soil that could be susceptible to surface erosion; and,
- Monitor all measures during the rainfall event and take corrective action where a potential for failure is identified.

The Contract Administrator shall document the status of the above-listed steps.

Responding to Failures

The Contractor will cease all construction-related work and focus on erosion and sediment control as required to stabilize the site where a failure has occurred or is imminent. The work shall be completed to the satisfaction of the Contract Administrator and any regulatory agencies having jurisdiction.

Any unexpected discharge of silt, sediment, or other deleterious substance shall be reported to the Municipality of Middlesex Centre within 24 hours. The Contractor is responsible for advising the contract administrator and promptly notifying the incident to the Spills Action Centre. Depending on the type of incident, water sampling and quality testing may be warranted to document the extent of the impact. Scoping for the required testing will depend on the incident report.

The Owner's Engineer will develop a restoration plan if significant long-term damage to aquatic habitat or property is suspected. Consultation with an ecologist and biologist may be required to confirm that the remedial measures are appropriate. Development of the initial restoration plan will begin within 24 hours of discovering sediment discharge. It will be implemented immediately following consultation and approval from the MECP, Lower Thames River Conservation Authority (UTRCA), and Municipality of Middlesex Centre. The plan will address the following:

- Removal and disposal of sediment deposited outside of the work limits; and
- Restoration of any areas disturbed through deposition or removal.

Reporting Schedule

The Contract Administrator shall prepare weekly erosion and sediment control monitoring reports/summaries for the duration of site grading and site servicing and submit them to the Municipality of Thames Center by April 1, July 1, and November 1 of each year until all works and services included in the plan are assumed.

The monitoring reports should document the status of the ESC Plan, any repairs, rainfall or pumping that has occurred since the last report, and any risks of failure that may be present.

Additionally, any failure of erosion and sediment control measures shall be reported as described in the contingency plan.

Construction Dewatering Requirements

Based on the Geotechnical Investigation prepared by LDS, shallow groundwater conditions are observed throughout the site. Therefore, where minor groundwater infiltration occurs within open excavations during construction, conventional sump pumping techniques are expected to be suitable for groundwater control.

Sediment controls should be incorporated into the construction of dewatering discharge outlets. These may be silt bags, constructed sediment traps, or other discharge water filtering methods. The Contractor will be responsible for regular maintenance, including sediment removal. Under no circumstances will dewatering effluent be discharged directly to the receiving watercourse without incorporating suitable measures to prevent sediment discharge or cause erosion or scouring at the waterway. Both the Contractor and the Contract Administrator will be responsible for monitoring the water quality leaving the site.

As an introductory guide, the water discharged at the site should have a turbidity level within 8 NTU of the background levels within the watercourse/municipal drain.

For construction dewatering efforts involving a sizeable daily pumping volume, additional sediment control or filtration measures, such as settlement tanks, may be implemented as part of the construction dewatering plan. The discharge and associated sediment controls will depend on the scope of work and the location of the excavation to be dewatered. Thus, the Owner's Engineer should review the dewatering plan in consultation with the Contractor and the Contract Administrator.

For projects requiring positive groundwater control with a removal rate of 50,000 to 400,000 litres per day, a submission to the Environmental Activity and Sector Registry (EASR) will be required. For construction dewatering activities with volumes over 400,000 litres per day, a Category 3 Permit to Take Water (PTTW) would be necessary for groundwater control. EASR submissions and PTTW applications are submitted to and approved by MECP according to Sections 34 and 98 of the Ontario Water Resources Act R.S.O. 1990 and Water Taking and Transfer Regulation O. Reg. 387/04.

The supporting documents to support an EASR submission or PTTW application should include calculations for the zone of influence and identify potential qualitative and quantitative impacts on the shallow groundwater table. Details regarding volume monitoring, water quality analyses and method/location of discharge water will also be required for either submission type. In addition, impacts on local natural features will need to be assessed considering the proposed construction dewatering plan.

Conclusion and Recommendations

The analysis completed by LDS yields the following conclusions:

- Post-development peak flow rates are attenuated to less than existing flow rates into the proposed municipal drain;
- Post-development peak flow rates are attenuated to less than the capacity of the pipe of the proposed municipal drain for up-to-the 25-year design storm event;
- Surface storage contained within the dry pond will be used to attenuate peak runoff flows;
- Surface runoff from all road and driveway surfaces is treated with an OGS device; and,
- Sediment transport from the site during construction will be minimized by implementing appropriate ESC measures.

We trust this submission will meet with your approval. However, if you have any questions or require further information, please do not hesitate to contact the undersigned.

Respectfully,

LDS CONSULTANTS INC.



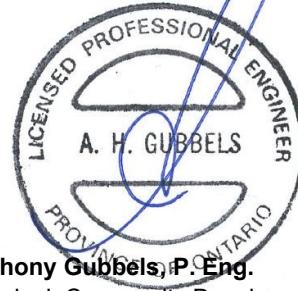
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anthony.gubbels@LDSc consultants.ca

Attachments:

Appendix A – FIGURES

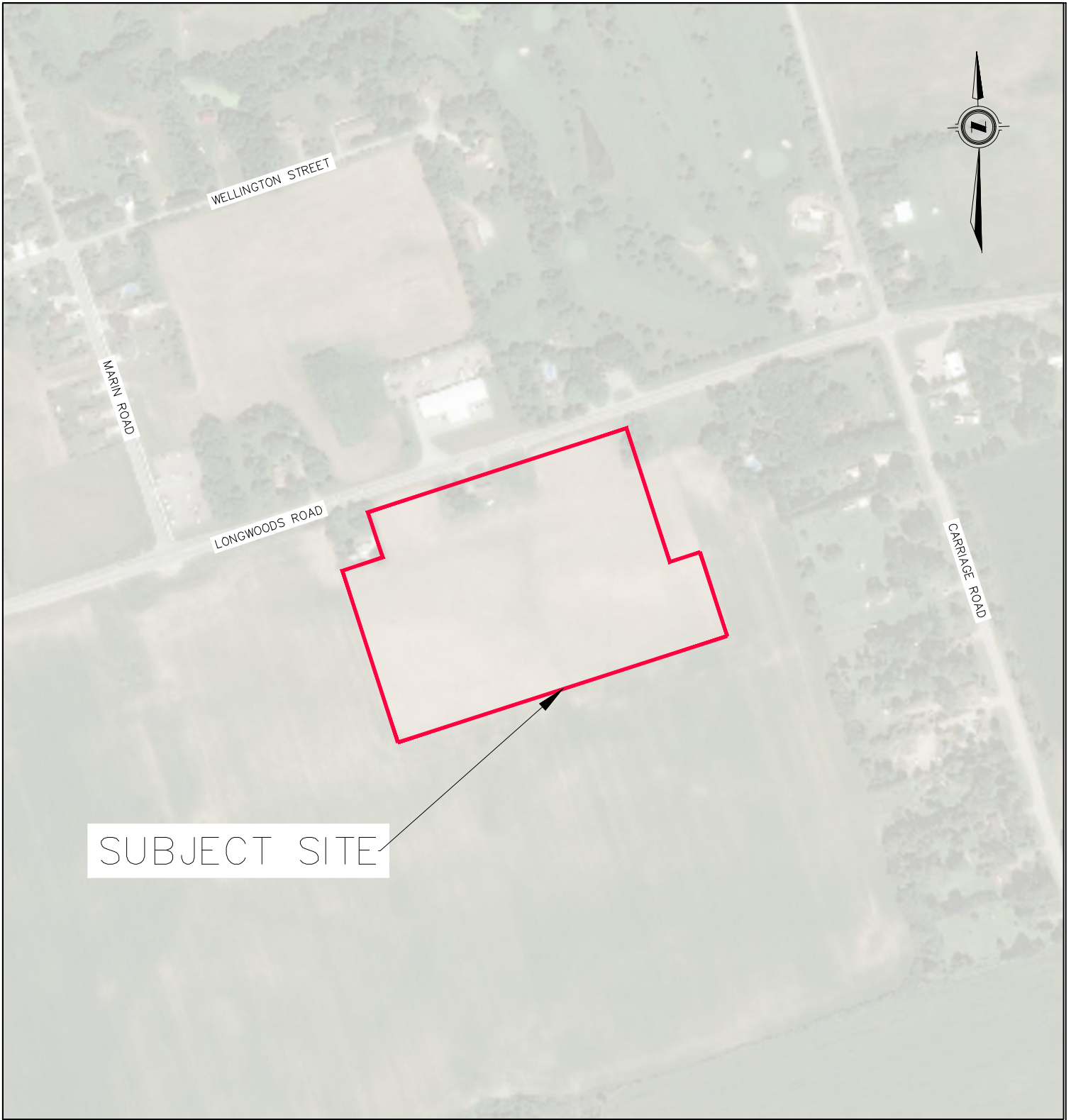
Appendix B – SWMHYMO HYDROLOGIC MODEL

Appendix C – SUPPORTING CALCULATIONS

Appendix D – OGS SIZING SHEET

APPENDIX A
FIGURES

Z:\LD-00318 - 10919 LONGWOODS ROAD\DETAIL DESIGN\REPORTS\SWM\DRAWINGS\LD-00318 - LOCATION PLAN.DWG
2023-03-15 9:13:31 AM by: LUKE JESSON



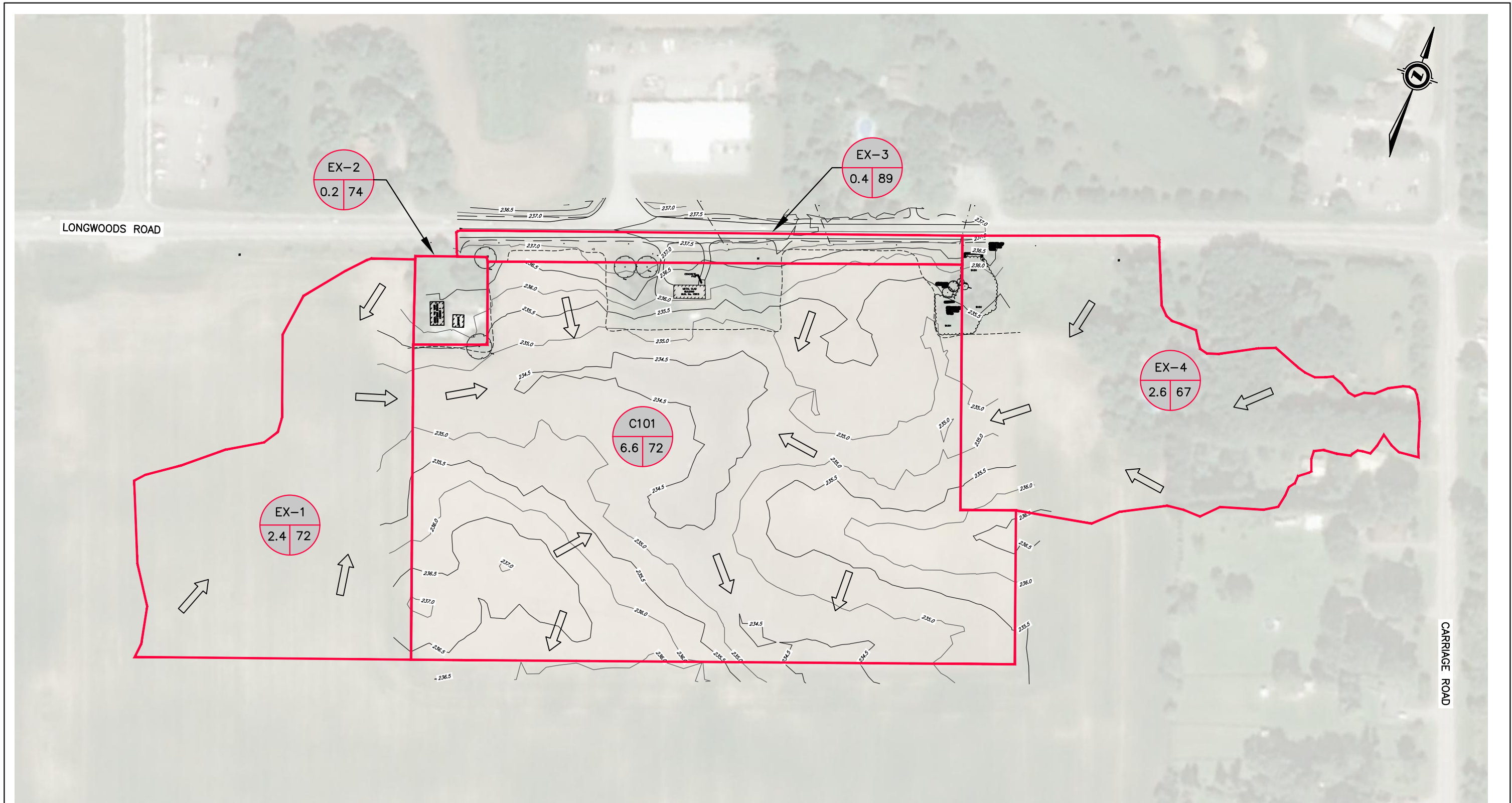
10919 LONGWOODS ROAD LIGHT INDUSTRIAL
10919 LONGWOODS ROAD INC.

LOCATION PLAN




PROJECT: LD-00318

SCALE: N.T.S.

FIGURE 1



LEGEND:

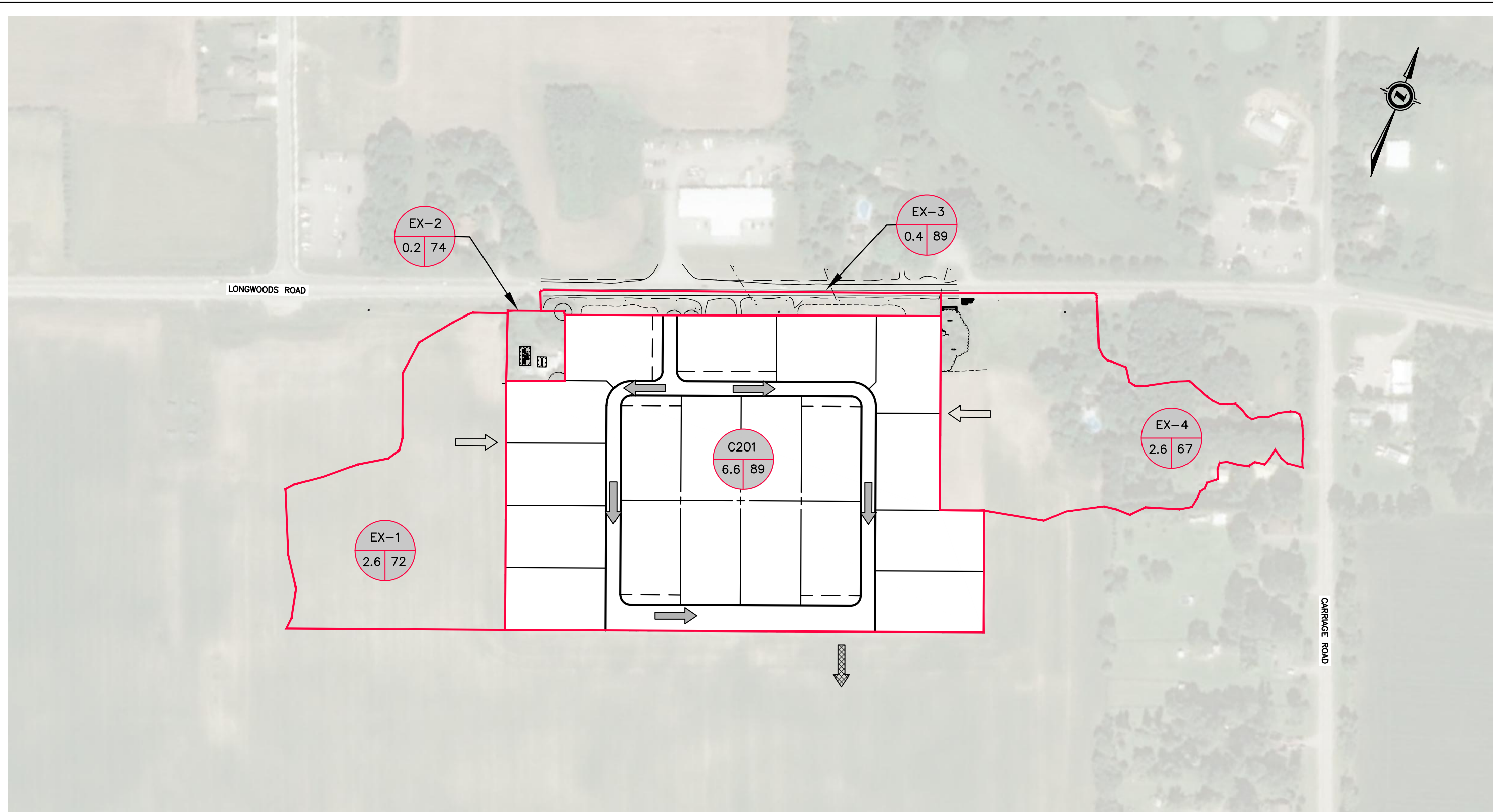
	EXISTING CATCHMENT AREA	STORM DRAINAGE AREA DATA:
	EXISTING OVERLAND FLOW PATH	CATCHMENT ID
		
		AREA (ha) CURVE NUMBER





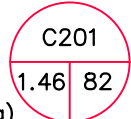

10919 LONGWOODS ROAD LIGHT INDUSTRIAL
10919 LONGWOODS ROAD INC.

EXISTING CONDITIONS DRAINAGE AREA PLAN

PROJECT: LD-00318 SCALE: N.T.S. FIGURE 2

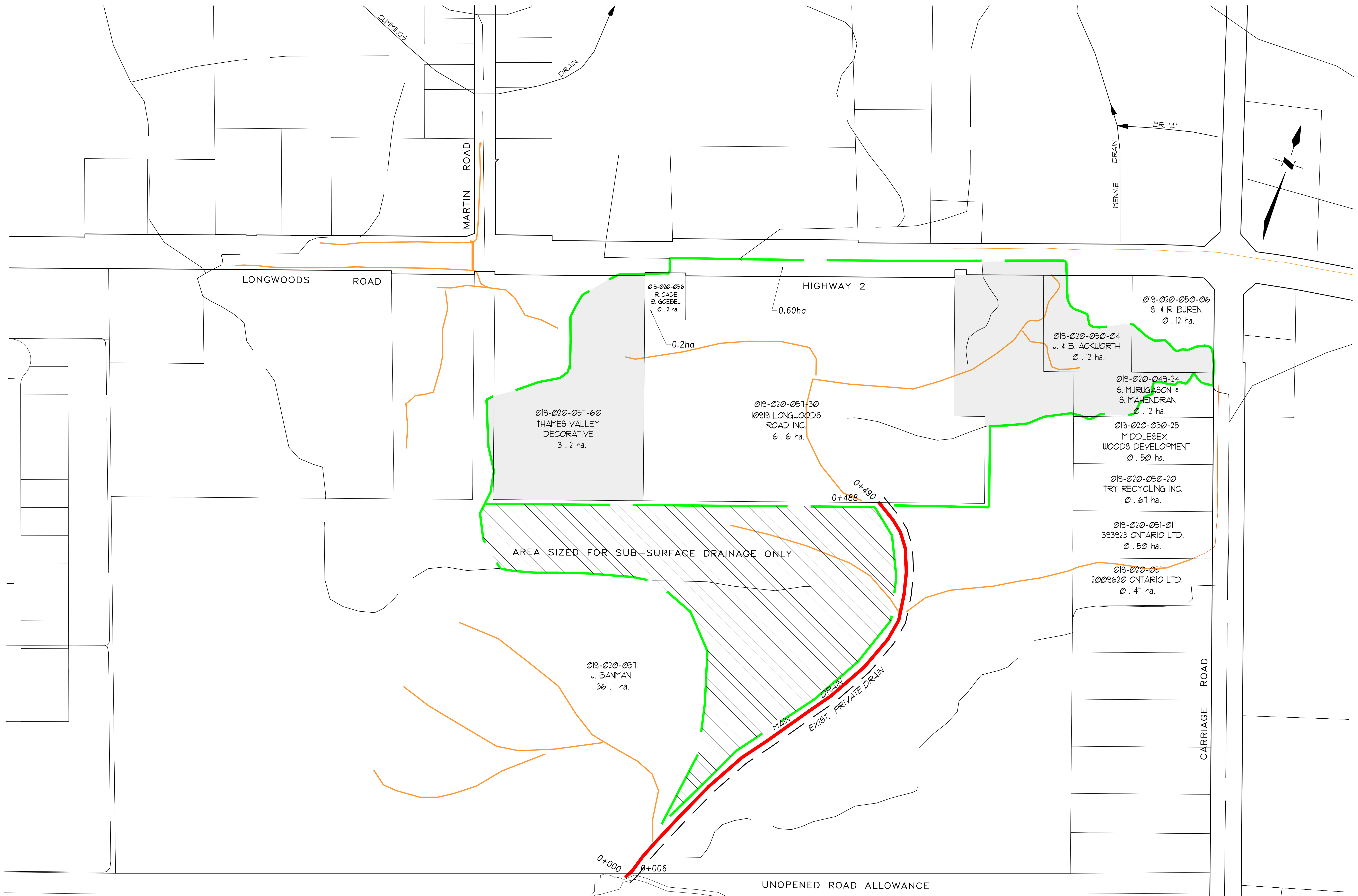


LEGEND:

	PROPOSED CATCHMENT AREA	STORM DRAINAGE AREA DATA:	
	PROPOSED OVERLAND FLOW ROUTE		CATCHMENT ID
	ULTIMATE OVERLAND FLOW ROUTE	AREA (ha)	CURVE NUMBER



10919 LONGWOODS ROAD LIGHT INDUSTRIAL
 10919 LONGWOODS ROAD INC.
POST-DEVELOPMENT DRAINAGE AREA PLAN
 PROJECT: LD-00318 SCALE: N.T.S. FIGURE 3



UNOPENED ROAD ALLOWANCE

APPENDIX B
SWMHYMO HYDROLOGIC MODEL

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00001> =====
00002>
00003> SSSSS W W M M H H Y Y M M OOO 999 999 =====
00004> S W W W M M M M H H Y Y M M M O O 9 9 9 9
00005> SSSSS W W M M M H H H H Y Y M M M O # 9 9 9 9 Ver 4.05
00006> S W W M M M H H Y Y M M O O 9999 9999 Sept 2011
00007> SSSSS W W M M H H Y Y M M OOO 9 9 9 =====
00008> # 4058874
00009> StormWater Management Hydrologic Model 999 999 =====
00010>
00011> *****
00012> ***** SWMMHYM Ver/4.05 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHYMO-83 and OTTHYMO-89. *****
00016> *****
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Ottawa, Ontario: (613) 836-3884 *****
00019> ***** Gatineau, Quebec: (819) 243-6858 *****
00020> ***** E-Mail: swmhyom@fsa.Com *****
00021> *****
00022> *****
00023> *****
00024> ***** Licensed user: Land Development Solutions *****
00025> ***** London SERIAL#:4058874 *****
00026> *****
00027> *****
00028> *****
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 105408 *****
00032> ***** Max. number of flow points : 105408 *****
00033> *****
00034> *****
00035> *****
00036> ***** D E T A I L E D O U T P U T *****
00037> *****
00038> ***** DATE: 2023-11-21 TIME: 14:53:34 RUN COUNTER: 000760 *****
00039> *****
00040> * Input filename: C:\SWMMHY-1\LD-00318\211123.dat *
00041> * Output filename: C:\SWMMHY-1\LD-00318\211123.out *
00042> * Summary filename: C:\SWMMHY-1\LD-00318\211123.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047> *****
00048> *****
00049> *****
00050> 001:0001-----
00051> # Project Name:[LONGWOODS INDUSTRIAL VLC, DELAWARE] Project Number: [LD-00318]
00052> # Date: [20-11-2023]
00053> # Modeller : [LJ]
00054> # Company : [LDS Consultants Inc.]
00055> # License # : [4058874]
00056> *****
00057> *****
00058> *****
00059> | START | Project dir.: C:\SWMMHY-1\LD-00318\
00060> | Rainfall dir.: C:\SWMMHY-1\LD-00318\
00061> | TZERO = .00 hrs on 0
00062> | METOUT= 2 (output = METRIC)
00063> | NRUN = 001
00064> | NSTORM= 0
00065> *****
00066> 001:0002-----
00067> # *****
00068> # *****
00069> # *****
00070> # *****
00071> # *****
00072> *****
00073> *****
00074> | CHICAGO STORM | IDF curve parameters: A= 724.690
00075> | Ptotal= 33.31 mm | B= 5.500
00076> | C= .800
00077> used in: INTENSITY = A / (t + B)^C
00078> *****
00079> | Duration of storm = 3.00 hrs
00080> | Storm time step = 5.00 min
00081> | Time to peak ratio = .33
00082> *****
00083> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00084> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00085> | .08 2.718 | .83 15.099 | 1.58 7.604 | 2.33 3.337
00086> | .17 2.963 | .92 35.585 | 1.67 6.700 | 2.42 3.350
00087> | .25 3.263 | 1.00 110.459 | 1.75 5.997 | 2.50 3.184
00088> | .33 3.638 | 1.08 45.982 | 1.83 5.435 | 2.58 3.034
00089> | .42 4.124 | 1.17 25.234 | 1.92 4.976 | 2.67 2.899
00090> | .50 4.778 | 1.25 17.203 | 2.00 4.593 | 2.75 2.777
00091> | .58 5.709 | 1.33 13.032 | 2.08 4.268 | 2.83 2.666
00092> | .67 7.143 | 1.42 10.502 | 2.17 3.990 | 2.92 2.564
00093> | .75 9.644 | 1.50 8.812 | 2.25 3.749 | 3.00 2.470
00094> *****
00095> *****
00096> 001:0003-----
00097> # *****
00098> # *****
00099> # *****
00100> # *****
00101> # *****
00102> *****
00103> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
00104> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00105> | U.H. Tp(hrs)= .330
00106> *****
00107> | Unit Hyd Qpeak (cms)= .274
00108> *****
00109> | PEAK FLOW (cms)= .027 (i)
00110> | TIME TO PEAK (hrs)= 1.517
00111> | RUNOFF VOLUME (mm)= 5.161
00112> | TOTAL RAINFALL (mm)= 33.307
00113> | RUNOFF COEFFICIENT = .155
00114> *****
00115> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00116> *****
00117> *****
00118> 001:0004-----
00119> # *****
00120> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00121> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
00122> | Total minor system capacity = .001 (cms)
00123> | Total major system storage [TMJSTO] = 99999. (cu.m.)
00124> *****
00125> | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00126> | (ha) (cms) (hrs) (mm) (cms)
00127> | TOTAL HYD. 01:EX-1 2.37 .027 1.517 5.161 .000
00128> *****
00129> | MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
00130> | MINOR SYST 03:toDRAI 2.37 .001 43.450 5.162 .000
00131> *****
00132> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00133> *****
00134> | Maximum MAJOR SYSTEM storage used = 114. (cu.m.)
00135> *****

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00136> -----
00137> 001:0005-----
00138> *****
00139> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
00140> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00141> | U.H. Tp(hrs)= .090
00142> *****
00143> | Unit Hyd Qpeak (cms)= .081
00144> *****
00145> | PEAK FLOW (cms)= .006 (i)
00146> | TIME TO PEAK (hrs)= 1.117
00147> | RUNOFF VOLUME (mm)= 6.816
00148> | TOTAL RAINFALL (mm)= 33.307
00149> | RUNOFF COEFFICIENT = .205
00150> *****
00151> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00152> *****
00153> *****
00154> 001:0006-----
00155> *****
00156> | CALIB STANDHYD | Area (ha)= .45
00157> | 04:EX-3 DT= 1.00 | Total Imp(%)= 65.00 Dir. Conn.(%)= 1.00
00158> *****
00159> | IMPERVIOUS PERVIOUS (i)
00160> | Surface Area (ha)= .29 .16
00161> | Dep. Storage (mm)= 2.00 8.00
00162> | Average Slope (%)= 1.00 20.00
00163> | Length (m)= 5.00 25.00
00164> | Mannings n = .013 .240
00165> *****
00166> | Max.eff.Inten.(mm/hr)= 110.46 135.03
00167> | over (min) 1.00 3.00
00168> | Storage Coeff. (min)= .41 (ii) 2.72 (ii)
00169> | Unit Hyd. Tpeak (min)= 1.00 3.00
00170> | Unit Hyd. peak (cms)= 1.55 .40
00171> *****
00172> | PEAK FLOW (cms)= .00 .04 .045 (iii)
00173> | TIME TO PEAK (hrs)= 1.00 1.02 1.017
00174> | RUNOFF VOLUME (mm)= 31.31 14.20 14.375
00175> | TOTAL RAINFALL (mm)= 33.31 33.31 33.307
00176> | RUNOFF COEFFICIENT = .94 .43 .432
00177> *****
00178> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00179> | CN* = 72.0 Ia = Dep. Storage (Above)
00180> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00181> | THAN THE STORAGE COEFFICIENT.
00182> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00183> *****
00184> *****
00185> 001:0007-----
00186> *****
00187> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
00188> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00189> | U.H. Tp(hrs)= .230
00190> *****
00191> | Unit Hyd Qpeak (cms)= .430
00192> *****
00193> | PEAK FLOW (cms)= .029 (i)
00194> | TIME TO PEAK (hrs)= 1.367
00195> | RUNOFF VOLUME (mm)= 4.258
00196> | TOTAL RAINFALL (mm)= 33.307
00197> | RUNOFF COEFFICIENT = .128
00198> *****
00199> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00200> *****
00201> *****
00202> 001:0008-----
00203> # *****
00204> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00205> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
00206> | Total minor system capacity = .001 (cms)
00207> | Total major system storage [TMJSTO] = 99999. (cu.m.)
00208> *****
00209> | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00210> | (ha) (cms) (hrs) (mm) (cms)
00211> | TOTAL HYD. 05:EX-4 2.59 .029 1.367 4.258 .000
00212> *****
00213> | MAJOR SYST 06:BANMAN .00 .000 .000 .000 .000
00214> | MINOR SYST 07:toDRAI 2.59 .001 39.250 4.258 .000
00215> *****
00216> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00217> *****
00218> | Maximum MAJOR SYSTEM storage used = 102. (cu.m.)
00219> *****
00220> *****
00221> 001:0009-----
00222> *****
00223> | CALIB NASHYD | Area (ha)= 6.64 Curve Number (CN)=72.00
00224> | 05:C101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00225> | U.H. Tp(hrs)= .310
00226> *****
00227> | Unit Hyd Qpeak (cms)= .818
00228> *****
00229> | PEAK FLOW (cms)= .077 (i)
00230> | TIME TO PEAK (hrs)= 1.483
00231> | RUNOFF VOLUME (mm)= 5.161
00232> | TOTAL RAINFALL (mm)= 33.307
00233> | RUNOFF COEFFICIENT = .155
00234> *****
00235> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00236> *****
00237> *****
00238> *****
00239> *****
00240> | ADD HYD (PRE ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00241> | (ha) (cms) (hrs) (mm) (cms)
00242> | ID1 01:EX-2 .19 .006 1.12 6.82 .000
00243> | +ID2 03:toDRAIN 2.37 .001 43.45 5.16 .000
00244> | +ID3 04:EX-3 .45 .045 1.02 14.38 .000
00245> | +ID4 05:C101 6.64 .077 1.48 5.16 .000
00246> | +ID5 07:toDRAIN 2.59 .001 39.25 4.26 .000
00247> *****
00248> | SUM 08:PRE 12.24 .089 1.45 5.33 .000
00249> *****
00250> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00251> *****
00252> *****
00253> *****
00254> # *****
00255> # *****
00256> # *****
00257> # *****
00258> # *****
00259> *****
00260> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
00261> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00262> | U.H. Tp(hrs)= .330
00263> *****
00264> | Unit Hyd Qpeak (cms)= .274
00265> *****
00266> | PEAK FLOW (cms)= .027 (i)
00267> | TIME TO PEAK (hrs)= 1.517
00268> | RUNOFF VOLUME (mm)= 5.161
00269> | TOTAL RAINFALL (mm)= 33.307
00270> | RUNOFF COEFFICIENT = .155
00271> *****

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00271> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00272>
00273>
00274>
00275> 001:0012-----
00276>
00277> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00278> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
00279> |-----| Total minor system capacity = .001 (cms)
00280> |-----| Total major system storage [TMJSTO] = 99999.(cu.m.)
00281>
00282> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00283> (ha) (cms) (hrs) (mm) (cms)
00284> TOTAL HYD. 01:EX-1 2.37 .027 1.517 5.161 .000
00285>
00286> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
00287> MINOR SYST 03:toDRAI 2.37 .001 43.450 5.162 .000
00288>
00289> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00290>
00291> Maximum MAJOR SYSTEM storage used = 114.(cu.m.)
00292>
00293>
00294> 001:0013-----
00295>
00296> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
00297> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
00298> |-----| U.H. Tp(hrs)= .090
00299>
00300> Unit Hyd Qpeak (cms)= .081
00301>
00302> PEAK FLOW (cms)= .006 (i)
00303> TIME TO PEAK (hrs)= 1.117 (mm)
00304> RUNOFF VOLUME (mm)= 6.816
00305> TOTAL RAINFALL (mm)= 33.307
00306> RUNOFF COEFFICIENT = .205
00307>
00308> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00309>
00310>
00311> 001:0014-----
00312>
00313> | CALIB STANDHYD | Area (ha)= .45
00314> | 04:EX-3 DT= 1.00 | Total Imp(%)= 65.00 Dir. Conn.(%)= 1.00
00315>
00316> IMPERVIOUS PERVIOUS (i)
00317> Surface Area (ha)= .29 .16
00318> Dep. Storage (mm)= 2.00 8.00
00319> Average Slope (%)= 1.00 20.00
00320> Length (m)= 5.00 25.00
00321> Mannings n = .013 .240
00322>
00323> Max.eff.Inten.(mm/hr)= 110.46 135.03
00324> over (min) 1.00 3.00
00325> Storage Coeff. (min)= .41 (ii) 2.72 (ii)
00326> Unit Hyd. Tpeak (min)= 1.00 3.00
00327> Unit Hyd. peak (cms)= 1.55 .40
00328>
00329> PEAK FLOW (cms)= .00 .04 *TOTALS*
00330> TIME TO PEAK (hrs)= 1.00 1.02 .045 (iii)
00331> RUNOFF VOLUME (mm)= 31.31 14.20 1.017
00332> TOTAL RAINFALL (mm)= 33.31 33.31 14.375
00333> RUNOFF COEFFICIENT = .94 .43 33.307
00334>
00335> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00336> CN* = 72.0 Ia = Dep. Storage (Above)
00337> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00338> THAN THE STORAGE COEFFICIENT.
00339> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00340>
00341>
00342> 001:0015-----
00343>
00344> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
00345> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
00346> |-----| U.H. Tp(hrs)= .230
00347>
00348> Unit Hyd Qpeak (cms)= .430
00349>
00350> PEAK FLOW (cms)= .029 (i)
00351> TIME TO PEAK (hrs)= 1.367
00352> RUNOFF VOLUME (mm)= 4.258
00353> TOTAL RAINFALL (mm)= 33.307
00354> RUNOFF COEFFICIENT = .128
00355>
00356> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00357>
00358>
00359> 001:0016-----
00360>
00361> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00362> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
00363> |-----| Total minor system capacity = .001 (cms)
00364> |-----| Total major system storage [TMJSTO] = 99999.(cu.m.)
00365>
00366> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00367> (ha) (cms) (hrs) (mm) (cms)
00368> TOTAL HYD. 05:EX-4 2.59 .029 1.367 4.258 .000
00369>
00370> MAJOR SYST 06:BANMAN .00 .000 .000 .000 .000
00371> MINOR SYST 07:toDRAI 2.59 .001 39.250 4.258 .000
00372>
00373> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00374>
00375> Maximum MAJOR SYSTEM storage used = 102.(cu.m.)
00376>
00377>
00378> 001:0017-----
00379>
00380> | CALIB STANDHYD | Area (ha)= 6.64
00381> | 05:C201 DT= 1.00 | Total Imp(%)= 90.00 Dir. Conn.(%)= 80.00
00382>
00383> IMPERVIOUS PERVIOUS (i)
00384> Surface Area (ha)= 5.98 .66
00385> Dep. Storage (mm)= 2.00 5.00
00386> Average Slope (%)= 1.00 2.00
00387> Length (m)= 50.00 25.00
00388> Mannings n = .013 .250
00389>
00390> Max.eff.Inten.(mm/hr)= 110.46 36.87
00391> over (min) 2.00 10.00
00392> Storage Coeff. (min)= 1.62 (ii) 9.55 (ii)
00393> Unit Hyd. Tpeak (min)= 2.00 10.00
00394> Unit Hyd. peak (cms)= .64 .12
00395>
00396> PEAK FLOW (cms)= 1.55 .04 *TOTALS*
00397> TIME TO PEAK (hrs)= 1.00 1.18 1.565 (iii)
00398> RUNOFF VOLUME (mm)= 31.31 8.47 26.740
00399> TOTAL RAINFALL (mm)= 33.31 33.31 33.307
00400> RUNOFF COEFFICIENT = .94 .25 .803
00401>
00402> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00403> CN* = 61.0 Ia = Dep. Storage (Above)
00404> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00405> THAN THE STORAGE COEFFICIENT.

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00406> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00407>
00408>
00409> 001:0018-----
00410>
00411> | ADD HYD (POST ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00412> |-----| (ha) (cms) (hrs) (mm) (cms)
00413> ID1 01:EX-2 .19 .006 1.12 6.82 .000
00414> +ID2 03:toDRAIN 2.37 .001 43.45 5.16 .000
00415> +ID3 04:EX-3 .45 .045 1.02 14.38 .000
00416> +ID4 05:C201 6.64 1.565 1.00 26.74 .000
00417> +ID5 07:toDRAIN 2.59 .001 39.25 4.26 .000
00418> =====
00419> SUM 09:POST 12.24 1.610 1.00 17.04 .000
00420>
00421> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00422>
00423>
00424> 001:0019-----
00425>
00426> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00427> | IN>09: (POST ) |
00428> | OUT<01: (SWMF ) | ===== OUTFLOW STORAGE TABLE =====
00429> |-----| OUTFLOW STORAGE OUTFLOW STORAGE
00430> (cms) (ha.m.) (cms) (ha.m.)
00431> .000 .0000E+00 | .079 .1497E+00
00432> .059 .1884E-01 | .080 .1683E+00
00433> .069 .338E-01 | .082 .1875E+00
00434> .070 .4889E-01 | .083 .2073E+00
00435> .072 .6439E-01 | .085 .2276E+00
00436> .073 .8041E-01 | .086 .2485E+00
00437> .075 .9639E-01 | .087 .2700E+00
00438> .076 .1140E+00 | .089 .4003E+00
00439> .078 .1316E+00 | .000 .0000E+00
00440>
00441> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00442> (ha) (cms) (hrs) (mm)
00443> INFLOW >09: (POST ) 12.24 1.610 1.000 17.041
00444> OUTFLOW<01: (SWMF ) 12.24 .078 2.083 17.041
00445> OVERFLOW<02: (000003) .00 .000 .000 .000
00446>
00447> TOTAL NUMBER OF SIMULATED OVERFLOWS = 0
00448> CUMULATIVE TIME OF OVERFLOWS (hours) = .00
00449> PERCENTAGE OF TIME OVERFLOWING (%) = .00
00450>
00451> PEAK FLOW REDUCTION [Qout/Qin](%) = 4.843
00452> TIME SHIFT OF PEAK FLOW (min) = 65.00
00453> MAXIMUM STORAGE USED (ha.m.) = 1315E+00
00454>
00455>
00456>
00457> 001:0020-----
00458>
00459> | DIVERT HYD |
00460> | INID=01 (SWMF ) |
00461>
00462> Outflow / Inflow Relationships
00463> Flow 03 + Flow 04 + Flow 05 = Total
00464> (cms) (cms) (cms) (cms)
00465> .000 .000 .000 .000
00466> .000 .059 .000 .059
00467> .007 .061 .000 .069
00468> .007 .063 .000 .070
00469> .008 .064 .000 .072
00470> .008 .065 .000 .073
00471> .008 .066 .000 .075
00472> .009 .068 .000 .076
00473> .009 .069 .000 .078
00474> .009 .070 .000 .079
00475> .009 .071 .000 .080
00476> .010 .072 .000 .082
00477> .010 .073 .000 .083
00478> .010 .074 .000 .085
00479> .010 .075 .000 .086
00480> .011 .077 .000 .087
00481> .011 .078 .210 .299
00482>
00483> NHYD AREA QPEAK TpeakDate_hh:mm R.V. NFE Wet(hrs)
00484> (ha) (cms) (mm) (mm)
00485> IDin = 01:SWMF 12.24 .078 No date 2:05 17.041 1 51.
00486>
00487> IDout= 03:Infillt 1.04 .009 No date 2:05 17.041 1 6.
00488> IDout= 04:Orific 11.20 .069 No date 2:05 17.041 1 51.
00489> IDout= 05:Emerge .00 .000 No date 0:00 .000 0 0.
00490>
00491> 001:0021-----
00492> *****
00493> ***** 5-year *****
00494> *****
00495> *****
00496> *****
00497> *****
00498> *****
00499> | CHICAGO STORM | IDF curve parameters: A=1330.310
00500> | Ptotal= 45.37 mm | B= 7.938
00501> |-----| C= .855
00502> used in: INTENSITY = A / (t + B)^C
00503>
00504> Duration of storm = 3.00 hrs
00505> Storm time step = 5.00 min
00506> Time to peak ratio = .33
00507>
00508> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00509> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00510> .08 3.038 | .83 21.781 | 1.58 10.023 | 2.33 4.120
00511> .17 3.356 | .92 53.191 | 1.67 8.657 | 2.42 3.868
00512> .25 3.752 | 1.00 149.041 | 1.75 7.613 | 2.50 3.646
00513> .33 4.257 | 1.08 68.677 | 1.83 6.790 | 2.58 3.449
00514> .42 4.925 | 1.17 37.777 | 1.92 6.127 | 2.67 3.273
00515> .50 5.845 | 1.25 25.123 | 2.00 5.581 | 2.75 3.114
00516> .58 7.190 | 1.33 18.497 | 2.08 5.125 | 2.83 2.971
00517> .67 9.326 | 1.42 14.506 | 2.17 4.739 | 2.92 2.841
00518> .75 13.171 | 1.50 11.874 | 2.25 4.407 | 3.00 2.722
00519>
00520>
00521> 001:0022-----
00522> *****
00523> *****
00524> ***** EXISTING CONDITIONS *****
00525> *****
00526> *****
00527>
00528> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
00529> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
00530> |-----| U.H. Tp(hrs)= .330
00531>
00532> Unit Hyd Qpeak (cms)= .274
00533>
00534> PEAK FLOW (cms)= .060 (i)
00535> TIME TO PEAK (hrs)= 1.483
00536> RUNOFF VOLUME (mm)= 10.255
00537> TOTAL RAINFALL (mm)= 45.366
00538> RUNOFF COEFFICIENT = .226
00539>
00540> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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00541>
00542> -----
00543> 001:0023-----
00544>
00545> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00546> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
00547> | TotalHyd 01:EX-1 | Total minor system capacity = .001 (cms)
00548> | TotalHyd 01:EX-1 | Total major system storage [TMJSTO] = 99999. (cu.m.)
00549>
00550> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00551> (ha) (cms) (hrs) (mm) (cms)
00552> TOTAL HYD. 01:EX-1 2.37 .060 1.483 10.255 .000
00553>
00554> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
00555> MINOR SYST 03:toDRAI 2.37 .001 85.350 10.257 .000
00556>
00557> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00558>
00559> Maximum MAJOR SYSTEM storage used = 234. (cu.m.)
00560>
00561> -----
00562> 001:0024-----
00563>
00564> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
00565> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
00566> | U.H. Tp(hrs)= .090
00567>
00568> Unit Hyd Qpeak (cms)= .081
00569>
00570> PEAK FLOW (cms)= .012 (i)
00571> TIME TO PEAK (hrs)= 1.117
00572> RUNOFF VOLUME (mm)= 12.571
00573> TOTAL RAINFALL (mm)= 45.366
00574> RUNOFF COEFFICIENT = .277
00575>
00576> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00577>
00578> -----
00579> 001:0025-----
00580>
00581> | CALIB STANDHYD | Area (ha)= .45
00582> | 04:EX-3 DT= 1.00 | Total Imp(%)= 65.00 Dir. Conn.(%)= 1.00
00583>
00584> IMPERVIOUS PERVIOUS (i)
00585> Surface Area (ha)= .29 .16
00586> Dep. Storage (mm)= 2.00 8.00
00587> Average Slope (%)= 1.00 20.00
00588> Length (m)= 5.00 25.00
00589> Mannings n = .013 .240
00590>
00591> Max.eff.Inten.(mm/hr)= 149.04 237.09
00592> over (min) 1.00 2.00
00593> Storage Coeff. (.36 (ii) 2.20 (ii)
00594> Unit Hyd. Tpeak (min)= 1.00 2.00
00595> Unit Hyd. peak (cms)= 1.59 .52
00596>
00597> PEAK FLOW (cms)= .00 .08 .085 (iii)
00598> TIME TO PEAK (hrs)= .98 1.02 1.000
00599> RUNOFF VOLUME (mm)= 43.37 23.36 23.560
00600> TOTAL RAINFALL (mm)= 45.37 45.37 45.366
00601> RUNOFF COEFFICIENT = .96 .51 .519
00602>
00603> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00604> CN* = 72.0 Ia = Dep. Storage (Above)
00605> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00606> THAN THE STORAGE COEFFICIENT.
00607> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00608>
00609> -----
00610> 001:0026-----
00611>
00612> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
00613> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
00614> | U.H. Tp(hrs)= .230
00615>
00616> Unit Hyd Qpeak (cms)= .430
00617>
00618> PEAK FLOW (cms)= .067 (i)
00619> TIME TO PEAK (hrs)= 1.333
00620> RUNOFF VOLUME (mm)= 8.594
00621> TOTAL RAINFALL (mm)= 45.366
00622> RUNOFF COEFFICIENT = .189
00623>
00624> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00625>
00626> -----
00627> 001:0027-----
00628>
00629> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00630> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
00631> | TotalHyd 05:EX-4 | Total minor system capacity = .001 (cms)
00632> | TotalHyd 05:EX-4 | Total major system storage [TMJSTO] = 99999. (cu.m.)
00633>
00634> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00635> (ha) (cms) (hrs) (mm) (cms)
00636> TOTAL HYD. 05:EX-4 2.59 .067 1.333 8.594 .000
00637>
00638> MAJOR SYST 06:BANNAN .00 .000 .000 .000 .000
00639> MINOR SYST 07:toDRAI 2.59 .001 78.217 8.595 .000
00640>
00641> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00642>
00643> Maximum MAJOR SYSTEM storage used = 214. (cu.m.)
00644>
00645> -----
00646> 001:0028-----
00647>
00648> | CALIB NASHYD | Area (ha)= 6.64 Curve Number (CN)=72.00
00649> | 05:C101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
00650> | U.H. Tp(hrs)= .310
00651>
00652> Unit Hyd Qpeak (cms)= .818
00653>
00654> PEAK FLOW (cms)= .175 (i)
00655> TIME TO PEAK (hrs)= 1.450
00656> RUNOFF VOLUME (mm)= 10.255
00657> TOTAL RAINFALL (mm)= 45.366
00658> RUNOFF COEFFICIENT = .226
00659>
00660> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00661>
00662> -----
00663> 001:0029-----
00664>
00665> | ADD HYD (PRE ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00666> (ha) (cms) (hrs) (mm) (cms)
00667> ID1 01:EX-2 .19 .012 1.12 12.57 .000
00668> +ID2 03:toDRAIN 2.37 .001 85.35 10.26 .000
00669> +ID3 04:EX-3 .45 .085 1.00 23.56 .000
00670> +ID4 05:C101 6.64 .175 1.45 10.26 .000
00671> +IDS 07:toDRAIN 2.59 .001 78.22 8.59 .000
00672>
00673> SUM 08:PRE 12.24 .194 1.42 10.43 .000
00674>
00675> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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00676>
00677> -----
00678> 001:0030-----
00679> *#-----
00680> *#-----
00681> *#-----
00682> *#-----
00683> *#-----
00684>
00685> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
00686> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
00687> | U.H. Tp(hrs)= .330
00688>
00689> Unit Hyd Qpeak (cms)= .274
00690>
00691> PEAK FLOW (cms)= .060 (i)
00692> TIME TO PEAK (hrs)= 1.483
00693> RUNOFF VOLUME (mm)= 10.255
00694> TOTAL RAINFALL (mm)= 45.366
00695> RUNOFF COEFFICIENT = .226
00696>
00697> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00698>
00699> -----
00700> 001:0031-----
00701>
00702> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00703> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
00704> | TotalHyd 01:EX-1 | Total minor system capacity = .001 (cms)
00705> | TotalHyd 01:EX-1 | Total major system storage [TMJSTO] = 99999. (cu.m.)
00706>
00707> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00708> (ha) (cms) (hrs) (mm) (cms)
00709> TOTAL HYD. 01:EX-1 2.37 .060 1.483 10.255 .000
00710>
00711> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
00712> MINOR SYST 03:toDRAI 2.37 .001 85.350 10.257 .000
00713>
00714> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00715>
00716> Maximum MAJOR SYSTEM storage used = 234. (cu.m.)
00717>
00718> -----
00719> 001:0032-----
00720>
00721> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
00722> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
00723> | U.H. Tp(hrs)= .090
00724>
00725> Unit Hyd Qpeak (cms)= .081
00726>
00727> PEAK FLOW (cms)= .012 (i)
00728> TIME TO PEAK (hrs)= 1.117
00729> RUNOFF VOLUME (mm)= 12.571
00730> TOTAL RAINFALL (mm)= 45.366
00731> RUNOFF COEFFICIENT = .277
00732>
00733> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00734>
00735> -----
00736> 001:0033-----
00737>
00738> | CALIB STANDHYD | Area (ha)= .45
00739> | 04:EX-3 DT= 1.00 | Total Imp(%)= 65.00 Dir. Conn.(%)= 1.00
00740>
00741> IMPERVIOUS PERVIOUS (i)
00742> Surface Area (ha)= .29 .16
00743> Dep. Storage (mm)= 2.00 8.00
00744> Average Slope (%)= 1.00 20.00
00745> Length (m)= 5.00 25.00
00746> Mannings n = .013 .240
00747>
00748> Max.eff.Inten.(mm/hr)= 149.04 237.09
00749> over (min) 1.00 2.00
00750> Storage Coeff. (min)= .36 (ii) 2.20 (ii)
00751> Unit Hyd. Tpeak (min)= 1.00 2.00
00752> Unit Hyd. peak (cms)= 1.59 .52
00753>
00754> PEAK FLOW (cms)= .00 .08 .085 (iii)
00755> TIME TO PEAK (hrs)= .98 1.02 1.000
00756> RUNOFF VOLUME (mm)= 43.37 23.36 23.560
00757> TOTAL RAINFALL (mm)= 45.37 45.37 45.366
00758> RUNOFF COEFFICIENT = .96 .51 .519
00759>
00760> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00761> CN* = 72.0 Ia = Dep. Storage (Above)
00762> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00763> THAN THE STORAGE COEFFICIENT.
00764> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00765>
00766> -----
00767> 001:0034-----
00768>
00769> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
00770> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
00771> | U.H. Tp(hrs)= .230
00772>
00773> Unit Hyd Qpeak (cms)= .430
00774>
00775> PEAK FLOW (cms)= .067 (i)
00776> TIME TO PEAK (hrs)= 1.333
00777> RUNOFF VOLUME (mm)= 8.594
00778> TOTAL RAINFALL (mm)= 45.366
00779> RUNOFF COEFFICIENT = .189
00780>
00781> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00782>
00783> -----
00784> 001:0035-----
00785>
00786> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00787> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
00788> | TotalHyd 05:EX-4 | Total minor system capacity = .001 (cms)
00789> | TotalHyd 05:EX-4 | Total major system storage [TMJSTO] = 99999. (cu.m.)
00790>
00791> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00792> (ha) (cms) (hrs) (mm) (cms)
00793> TOTAL HYD. 05:EX-4 2.59 .067 1.333 8.594 .000
00794>
00795> MAJOR SYST 06:BANNAN .00 .000 .000 .000 .000
00796> MINOR SYST 07:toDRAI 2.59 .001 78.217 8.595 .000
00797>
00798> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00799>
00800> Maximum MAJOR SYSTEM storage used = 214. (cu.m.)
00801>
00802> -----
00803> 001:0036-----
00804>
00805> | CALIB STANDHYD | Area (ha)= 6.64
00806> | 05:C201 DT= 1.00 | Total Imp(%)= 90.00 Dir. Conn.(%)= 80.00
00807>
00808> IMPERVIOUS PERVIOUS (i)
00809> Surface Area (ha)= 5.98 .66
00810> Dep. Storage (mm)= 2.00 5.00

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00811> Average Slope (%) = 1.00 2.00
00812> Length (m) = 50.00 25.00
00813> Mannings n = .013 .250
00814>
00815> Max.eff.Inten.(mm/hr)= 149.04 73.97
00816> over (min) 1.00 7.00
00817> Storage Coeff. (min)= 1.44 (ii) 7.44 (iii)
00818> Unit Hyd. Tpeak (min)= 1.00 7.00
00819> Unit Hyd. peak (cms)= .85 .16
00820>
00821> PEAK FLOW (cms)= 2.16 .09 *TOTALS*
00822> TIME TO PEAK (hrs)= 1.00 1.12 1.000
00823> RUNOFF VOLUME (mm)= 43.37 14.81 37.65
00824> TOTAL RAINFALL (mm)= 45.37 45.37 45.366
00825> RUNOFF COEFFICIENT = .96 .33 .830
00826>
00827> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00828> CN* = 61.0 Ia = Dep. Storage (Above)
00829> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00830> THAN THE STORAGE COEFFICIENT.
00831> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00832>
00833>
00834> 001:0037-----
00835>
00836> | ADD HYD (POST ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00837> |-----|-----| (ha) (cms) (hrs) (mm) (cms)
00838> | ID1 01:EX-2 | .19 .012 1.12 12.57 .000
00839> | +ID2 03:toDRAIN | 2.37 .001 85.35 10.26 .000
00840> | +ID3 04:EX-3 | .45 .085 1.00 23.56 .000
00841> | +ID4 05:C201 | 6.64 2.194 1.00 37.66 .000
00842> | +IDS 07:toDRAIN | 2.59 .001 78.22 8.59 .000
00843> |-----|-----|
00844> | SUM 09:POST | 12.24 2.287 1.00 25.29 .000
00845>
00846> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00847>
00848>
00849> 001:0038-----
00850>
00851> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00852> | IN>09: (POST ) |
00853> | OUT<01: (SWMF ) |
00854> |-----|-----|
00855> | OUTFLOW STORAGE | OUTFLOW STORAGE
00856> | (cms) (ha.m.) | (cms) (ha.m.)
00857> | .000 .0000E+00 | .079 .1497E+00
00858> | .059 .1884E-01 | .080 .1683E+00
00859> | .069 .3388E-01 | .082 .1875E+00
00860> | .070 .4889E-01 | .083 .2073E+00
00861> | .072 .6439E-01 | .085 .2276E+00
00862> | .073 .8041E-01 | .086 .2485E+00
00863> | .075 .9693E-01 | .087 .2700E+00
00864> | .076 .1140E+00 | .089 .4003E+00
00865> | .078 .1316E+00 | .000 .0000E+00
00866>
00867> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00868> (ha) (cms) (hrs) (mm)
00869> INFLOW<09: (POST ) 12.24 2.287 1.000 25.294
00870> OUTFLOW<01: (SWMF ) 12.24 .083 2.250 25.294
00871> OVERFLOW<02: (000003) .00 .000 .000 .000
00872>
00873> TOTAL NUMBER OF SIMULATED OVERFLOWS = 0
00874> CUMULATIVE TIME OF OVERFLOWS (hours) = .00
00875> PERCENTAGE OF TIME OVERFLOWING (%) = .00
00876>
00877> PEAK FLOW REDUCTION [Qout/Qin] (%) = 3.620
00878> TIME SHIFT OF PEAK FLOW (min) = 75.00
00879> MAXIMUM STORAGE USED (ha.m.) = 2028E+00
00880>
00881>
00882> 001:0039-----
00883>
00884> | DIVERT HYD |
00885> | INID=01 (SWMF ) |
00886> |-----|-----|
00887> | Outflow / Inflow Relationships
00888> | Flow 03 + Flow 04 + Flow 05 = Total
00889> | (cms) (cms) (cms) (cms)
00890> | .000 .000 .000 .000
00891> | .000 .059 .000 .059
00892> | .007 .061 .000 .069
00893> | .007 .063 .000 .070
00894> | .008 .064 .000 .072
00895> | .008 .065 .000 .073
00896> | .008 .066 .000 .075
00897> | .009 .068 .000 .076
00898> | .009 .069 .000 .078
00899> | .009 .070 .000 .079
00900> | .009 .071 .000 .080
00901> | .010 .072 .000 .082
00902> | .010 .073 .000 .083
00903> | .010 .074 .000 .085
00904> | .010 .075 .000 .086
00905> | .011 .077 .000 .087
00906> | .011 .078 .210 .299
00907>
00908> NHYD AREA QPEAK TpeakDate_hh:mm R.V. NFE WetHrs
00909> (ha) (cms) (cms) (hrs) (mm) (hrs)
00910> | Idin = 01:SWMF | 12.24 .083 No_date 2:15 25.294 1 93.
00911> |-----|-----|
00912> | Idout= 03:Infil | 1.07 .010 No_date 2:15 25.294 1 9.
00913> | Idout= 04:Orific | 11.16 .073 No_date 2:15 25.294 1 93.
00914> | Idout= 05:Emerg | 00 .000 No_date 0:00 .000 0 0.
00915>
00916> 001:0040-----
00917>
00918>
00919> *#
00920> *# 10-year
00921> *# =====
00922> *#
00923>
00924> | CHICAGO STORM | IDF curve parameters: A=1497.190
00925> | Ptotal= 52.59 mm | B= 7.188
00926> | C= .850
00927>
00928> used in: INTENSITY = A / (t + B) ^ C
00929>
00930> Duration of storm = 3.00 hrs
00931> Storm time step = 5.00 min
00932> Time to peak ratio = .33
00933>
00934> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00935> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00936> .07 3.907 | .92 60.893 | 1.67 9.886 | 2.42 4.490
00937> .25 4.357 | 1.00 178.745 | 1.75 8.714 | 2.50 4.237
00938> .33 4.931 | 1.18 79.026 | 1.83 7.789 | 2.58 4.013
00939> .42 5.686 | 1.17 42.842 | 1.92 7.043 | 2.67 3.812
00940> .50 6.725 | 1.25 28.407 | 2.00 6.428 | 2.75 3.631
00941> .58 8.239 | 1.33 20.928 | 2.08 5.913 | 2.83 3.467
00942> .67 10.636 | 1.42 16.445 | 2.17 5.476 | 2.92 3.318
00943> .75 14.947 | 1.50 13.493 | 2.25 5.101 | 3.00 3.182
00944>
00945>

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00946> 001:0041-----
00947> *#
00948> *#
00949> *# EXISTING CONDITIONS
00950> *#
00951> *#
00952>
00953> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
00954> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
00955> |-----|-----|
00956> | U.H. Tp (hrs)= .330
00957>
00958> Unit Hyd Qpeak (cms)= .274
00959>
00960> PEAK FLOW (cms)= .083 (i)
00961> TIME TO PEAK (hrs)= 1.467
00962> RUNOFF VOLUME (mm)= 13.868
00963> TOTAL RAINFALL (mm)= 52.590
00964> RUNOFF COEFFICIENT = .264
00965>
00966> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00967>
00968> 001:0042-----
00969>
00970> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00971> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
00972> |-----|-----|
00973> | Total minor system capacity = .001 (cms)
00974> | Total major system storage [TMJSTO] = 99999. (cu.m.)
00975>
00976> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00977> (ha) (cms) (hrs) (mm) (cms)
00978> TOTAL HYD. 01:EX-1 2.37 .083 1.467 13.868 .000
00979>
00980> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
00981> MINOR SYST 03:toDRAI 2.37 .001 115.050 13.867 .000
00982>
00983> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00984>
00985> Maximum MAJOR SYSTEM storage used = 319. (cu.m.)
00986>
00987> 001:0043-----
00988>
00989> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
00990> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
00991> |-----|-----|
00992> | U.H. Tp (hrs)= .090
00993>
00994> Unit Hyd Qpeak (cms)= .081
00995>
00996> PEAK FLOW (cms)= .017 (i)
00997> TIME TO PEAK (hrs)= 1.100
00998> RUNOFF VOLUME (mm)= 16.551
00999> TOTAL RAINFALL (mm)= 52.590
01000> RUNOFF COEFFICIENT = .315
01001>
01002> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01003>
01004> 001:0044-----
01005>
01006> | CALIB STANDHYD | Area (ha)= .45
01007> | 04:EX-3 DT= 1.00 | Total Imp (%) = 65.00 Dir. Conn. (%) = 1.00
01008>
01009> IMPERVIOUS PERVIOUS (i)
01010> Surface Area (ha)= .29 .16
01011> Dep. Storage (mm)= 2.00 8.00
01012> Average Slope (%) = 1.00 20.00
01013> Length (m) = 5.00 25.00
01014> Mannings n = .013 .240
01015>
01016> Max.eff.Inten.(mm/hr)= 178.75 311.11
01017> over (min) 1.00 2.00
01018> Storage Coeff. (min)= .34 (ii) 1.99 (ii)
01019> Unit Hyd. Tpeak (min)= 1.00 2.00
01020> Unit Hyd. peak (cms)= 1.61 .56
01021>
01022> *TOTALS*
01023> PEAK FLOW (cms)= .00 .11 .116 (iii)
01024> TIME TO PEAK (hrs)= .98 1.00 1.000
01025> RUNOFF VOLUME (mm)= 50.59 29.24 29.454
01026> TOTAL RAINFALL (mm)= 52.59 52.59 52.590
01027> RUNOFF COEFFICIENT = .96 .56 .560
01028>
01029> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01030> CN* = 72.0 Ia = Dep. Storage (Above)
01031> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01032> THAN THE STORAGE COEFFICIENT.
01033> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01034>
01035> 001:0045-----
01036>
01037> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
01038> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01039> |-----|-----|
01040> | U.H. Tp (hrs)= .230
01041>
01042> Unit Hyd Qpeak (cms)= .430
01043>
01044> PEAK FLOW (cms)= .094 (i)
01045> TIME TO PEAK (hrs)= 1.317
01046> RUNOFF VOLUME (mm)= 11.717
01047> TOTAL RAINFALL (mm)= 52.590
01048> RUNOFF COEFFICIENT = .223
01049>
01050> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01051>
01052> 001:0046-----
01053>
01054> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01055> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
01056> |-----|-----|
01057> | Total minor system capacity = .001 (cms)
01058> | Total major system storage [TMJSTO] = 99999. (cu.m.)
01059>
01060> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01061> (ha) (cms) (hrs) (mm) (cms)
01062> TOTAL HYD. 05:EX-4 2.59 .094 1.317 11.717 .000
01063>
01064> MAJOR SYST 06:BARMAN .00 .000 .000 .000 .000
01065> MINOR SYST 07:toDRAI 2.59 .001 106.283 11.717 .000
01066>
01067> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01068>
01069> Maximum MAJOR SYSTEM storage used = 295. (cu.m.)
01070>
01071> 001:0047-----
01072>
01073> | CALIB NASHYD | Area (ha)= 6.64 Curve Number (CN)=72.00
01074> | 05:IC101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01075> |-----|-----|
01076> | U.H. Tp (hrs)= .310
01077>
01078> Unit Hyd Qpeak (cms)= .818
01079>
01080> PEAK FLOW (cms)= .242 (i)
01081> TIME TO PEAK (hrs)= 1.433

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01081> RUNOFF VOLUME (mm)= 13.868
01082> TOTAL RAINFALL (mm)= 52.590
01083> RUNOFF COEFFICIENT = .264
01084>
01085> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01086>
01087>
01088> 001:0048-----
01089>
01090> | ADD HYD (PRE ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01091> | (ha) (cms) (hrs) (mm) (cms)
01092> | ID1 01:EX-2 .19 .017 1.10 16.55 .000
01093> | +ID2 03:toDRAIN 2.37 .001 115.05 13.97 .000
01094> | +ID3 04:EX-3 .45 .116 1.00 29.45 .000
01095> | +ID4 05:C101 6.64 .242 1.43 13.87 .000
01096> | +ID5 07:toDRAIN 2.59 .001 106.28 11.72 .000
01097>
01098> SUM 08:PRE 12.24 .266 1.42 14.03 .000
01099>
01100> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01101>
01102>
01103> 001:0049-----
01104> *#*****
01105> *# PROPOSED CONDITIONS
01106> *# *****
01107> *# *****
01108> *# *****
01109>
01110> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
01111> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01112> | U.H. Tp(hrs)= .330
01113>
01114> Unit Hyd Qpeak (cms)= .274
01115>
01116> PEAK FLOW (cms)= .083 (i)
01117> TIME TO PEAK (hrs)= 1.467
01118> RUNOFF VOLUME (mm)= 13.868
01119> TOTAL RAINFALL (mm)= 52.590
01120> RUNOFF COEFFICIENT = .264
01121>
01122> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01123>
01124>
01125> 001:0050-----
01126>
01127> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01128> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
01129> | Total minor system capacity [MINLET] = .001 (cms)
01130> | Total major system storage [TMJSTO] = 99999. (cu.m.)
01131>
01132> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01133> | (ha) (cms) (hrs) (mm) (cms)
01134> | TOTAL HYD. 01:EX-1 2.37 .083 1.467 13.868 .000
01135>
01136> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
01137> MINOR SYST 03:toDRAI 2.37 .001 115.05 13.867 .000
01138>
01139> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01140>
01141> Maximum MAJOR SYSTEM storage used = 319. (cu.m.)
01142>
01143>
01144> 001:0051-----
01145>
01146> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
01147> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
01148> | U.H. Tp(hrs)= .090
01149>
01150> Unit Hyd Qpeak (cms)= .081
01151>
01152> PEAK FLOW (cms)= .017 (i)
01153> TIME TO PEAK (hrs)= 1.100
01154> RUNOFF VOLUME (mm)= 16.551
01155> TOTAL RAINFALL (mm)= 52.590
01156> RUNOFF COEFFICIENT = .315
01157>
01158> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01159>
01160>
01161> 001:0052-----
01162>
01163> | CALIB STANDHYD | Area (ha)= .45
01164> | 04:EX-3 DT= 1.00 | Total Imp(%)= 65.00 Dir. Conn.(%)= 1.00
01165>
01166> IMPERVIOUS PERVIOUS (i)
01167> | Surface Area (ha)= .29 .16
01168> | Dep. Storage (mm)= 2.00 8.00
01169> | Average Slope (%)= 1.00 20.00
01170> | Length (m)= 5.00 25.00
01171> | Mannings n = .013 .240
01172>
01173> Max.eff.Inten.(mm/hr)= 178.75 311.11
01174> | over (min) 1.00 2.00
01175> | Storage Coeff. (min)= .34 (ii) 1.99 (ii)
01176> | Unit Hyd. Tpeak (min)= 1.00 2.00
01177> | Unit Hyd. peak (cms)= 1.61 .56
01178>
01179> PEAK FLOW (cms)= .00 .11 *TOTALS*
01180> | TIME TO PEAK (hrs)= .98 1.00 .116 (iii)
01181> | RUNOFF VOLUME (mm)= 50.59 29.24 29.454
01182> | TOTAL RAINFALL (mm)= 52.59 52.59 52.590
01183> | RUNOFF COEFFICIENT = .96 .56 .560
01184>
01185> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01186> CN* = 72.0 Ia = Dep. Storage (Above)
01187> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01188> THAN THE STORAGE COEFFICIENT.
01189> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01190>
01191>
01192> 001:0053-----
01193>
01194> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
01195> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01196> | U.H. Tp(hrs)= .230
01197>
01198> Unit Hyd Qpeak (cms)= .430
01199>
01200> PEAK FLOW (cms)= .094 (i)
01201> TIME TO PEAK (hrs)= 1.317
01202> RUNOFF VOLUME (mm)= 11.717
01203> TOTAL RAINFALL (mm)= 52.590
01204> RUNOFF COEFFICIENT = .223
01205>
01206> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01207>
01208>
01209> 001:0054-----
01210>
01211> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01212> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
01213> | Total minor system capacity [MINLET] = .001 (cms)
01214> | Total major system storage [TMJSTO] = 99999. (cu.m.)
01215>

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01216> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01217> | (ha) (cms) (hrs) (mm) (cms)
01218> TOTAL HYD. 05:EX-4 2.59 .094 1.317 11.717 .000
01219>
01220> MAJOR SYST 06:BANMAN .00 .000 .000 .000 .000
01221> MINOR SYST 07:toDRAI 2.59 .001 106.283 11.717 .000
01222>
01223> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01224>
01225> Maximum MAJOR SYSTEM storage used = 295. (cu.m.)
01226>
01227>
01228> 001:0055-----
01229>
01230> | CALIB STANDHYD | Area (ha)= 6.64
01231> | 05:C201 DT= 1.00 | Total Imp(%)= 90.00 Dir. Conn.(%)= 80.00
01232>
01233> IMPERVIOUS PERVIOUS (i)
01234> | Surface Area (ha)= 5.98 .66
01235> | Dep. Storage (mm)= 2.00 5.00
01236> | Average Slope (%)= 1.00 2.00
01237> | Length (m)= 50.00 25.00
01238> | Mannings n = .013 .250
01239>
01240> Max.eff.Inten.(mm/hr)= 178.75 100.19
01241> | over (min) 1.00 7.00
01242> | Storage Coeff. (min)= 1.34 (ii) 6.66 (ii)
01243> | Unit Hyd. Tpeak (min)= 1.00 7.00
01244> | Unit Hyd. peak (cms)= .90 .17
01245>
01246> PEAK FLOW (cms)= 2.60 .13 *TOTALS*
01247> | TIME TO PEAK (hrs)= 1.00 1.10 1.000
01248> | RUNOFF VOLUME (mm)= 50.59 19.11 44.294
01249> | TOTAL RAINFALL (mm)= 52.59 52.59 52.590
01250> | RUNOFF COEFFICIENT = .96 .36 .842
01251>
01252> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01253> CN* = 61.0 Ia = Dep. Storage (Above)
01254> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01255> THAN THE STORAGE COEFFICIENT.
01256> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01257>
01258>
01259> 001:0056-----
01260>
01261> | ADD HYD (POST ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01262> | (ha) (cms) (hrs) (mm) (cms)
01263> | ID1 01:EX-2 2.37 .001 115.05 13.87 .000
01264> | +ID2 03:toDRAIN 2.37 .001 115.05 13.87 .000
01265> | +ID3 04:EX-3 .45 .116 1.00 29.45 .000
01266> | +ID4 05:C201 6.64 2.653 1.00 44.29 .000
01267> | +ID5 07:toDRAIN 2.59 .001 106.28 11.72 .000
01268>
01269> SUM 09:POST 12.24 2.779 1.00 30.53 .000
01270>
01271> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01272>
01273>
01274> 001:0057-----
01275>
01276> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01277> | IN>09: (POST ) |
01278> | OUT<01: (SWMF ) |
01279>
01280> OUTFLOW STORAGE TABLE
01281> | (cms) (ha.m.) | (cms) (ha.m.)
01282> | .000 .0000E+00 | .079 .1497E+00
01283> | .059 .1884E-01 | .080 .1683E+00
01284> | .069 .3388E-01 | .082 .1875E+00
01285> | .070 .4889E-01 | .083 .2073E+00
01286> | .072 .6439E-01 | .085 .2266E+00
01287> | .073 .8041E-01 | .086 .2485E+00
01288> | .075 .9693E-01 | .087 .2700E+00
01289> | .076 .1140E+00 | .089 .4003E+00
01290> | .078 .1316E+00 | .000 .0000E+00
01291>
01292> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01293> | (ha) (cms) (hrs) (mm)
01294> | INFLOW>09: (POST ) 12.24 2.779 1.000 30.531
01295> | OUTFLOW<01: (SWMF ) 12.24 .086 2.383 30.531
01296> | OVERFLOW<02: (000003) .00 .000 .000 .000
01297>
01298> TOTAL NUMBER OF SIMULATED OVERFLOWS = 0
01299> TIME SHIFT OF PEAK FLOW (min)= .00
01300> PERCENTAGE OF TIME OVERFLOWING (%)= .00
01301>
01302> PEAK FLOW REDUCTION [Qout/Qin](%)= 3.089
01303> CUMULATIVE TIME OF OVERFLOWS (hours)= .00
01304> MAXIMUM STORAGE USED (ha.m.)=.2452E+00
01305>
01306>
01307> 001:0058-----
01308>
01309> | DIVERT HYD |
01310> | INID=01 (SWMF ) |
01311>
01312> Outflow / Inflow Relationships
01313> | Flow 03 + Flow 04 + Flow 05 = Total
01314> | (cms) (cms) (cms) (cms)
01315> | .000 .000 .000 .000
01316> | .000 .059 .000 .059
01317> | .007 .061 .000 .069
01318> | .007 .063 .000 .070
01319> | .008 .064 .000 .072
01320> | .008 .065 .000 .073
01321> | .008 .066 .000 .075
01322> | .009 .068 .000 .076
01323> | .009 .069 .000 .078
01324> | .009 .070 .000 .079
01325> | .009 .071 .000 .080
01326> | .010 .072 .000 .082
01327> | .010 .073 .000 .083
01328> | .010 .074 .000 .085
01329> | .010 .075 .000 .086
01330> | .011 .077 .000 .087
01331> | .011 .078 .210 .299
01332>
01333> NHYD AREA QPEAK TpeakDate_hh:mm R.V. NFE Wethrs
01334> | (ha) (cms) (hrs) (mm) (hrs)
01335> | Idin = 01:SWMF 12.24 .086 No date 2:23 30.531 1 123.
01336>
01337> | Idout= 03:Infiltr 1.07 .010 No date 2:23 30.531 1 11.
01338> | Idout= 04:Orific 11.14 .075 No date 2:23 30.531 1 123.
01339> | Idout= 05:Emerg 0.00 .000 No date 0:00 .000 0 0.
01340>
01341> 001:0059-----
01342> *#*****
01343> *# *****
01344> *# 25-year
01345> *# *****
01346> *# *****
01347> *#*****
01348>
01349> | CHICAGO STORM | IDF curve parameters: A=1455.000
01350> | Ptotal= 60.37 mm | B= 5.000

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01351> ----- C= .820
01352> used in: INTENSITY = A / (t + B) ^ C
01353>
01354> Duration of storm = 3.00 hrs
01355> Storm time step = 5.00 min
01356> Time to peak ratio = .33
01357>
01358> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01359> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01360> .08 4.464 | .83 26.338 | 1.58 12.906 | 2.33 5.854
01361> .17 4.878 | .92 65.076 | 1.67 11.320 | 2.42 5.536
01362> .25 5.387 | 1.00 220.223 | 1.75 10.095 | 2.50 5.253
01363> .33 6.027 | 1.08 85.162 | 1.83 9.119 | 2.58 4.999
01364> .42 6.858 | 1.17 45.078 | 1.92 8.324 | 2.67 4.771
01365> .50 7.983 | 1.25 30.164 | 2.00 7.664 | 2.75 4.564
01366> .58 9.594 | 1.33 22.579 | 2.08 7.106 | 2.83 4.376
01367> .67 12.097 | 1.42 18.039 | 2.17 6.629 | 2.92 4.204
01368> .75 16.515 | 1.50 15.035 | 2.25 6.216 | 3.00 4.046
01369>
01370> -----
01371> 001:0060
01372> *#-----
01373> *# EXISTING CONDITIONS
01374> *#
01375> *#
01376> *#-----
01377> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
01378> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01379> | U.H. Tp (hrs)= .330
01380>
01381> Unit Hyd Qpeak (cms)= .274
01382>
01383> PEAK FLOW (cms)= .109 (i)
01384> TIME TO PEAK (hrs)= 1.433
01385> RUNOFF VOLUME (mm)= 18.147
01386> TOTAL RAINFALL (mm)= 60.373
01387> RUNOFF COEFFICIENT = .301
01388>
01389> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01390>
01391> -----
01392> 001:0061
01393> *#-----
01394> *# COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01395> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
01396> | Total minor system capacity = .001 (cms)
01397> | Total major system storage [TMJSTO] = 99999. (cu.m.)
01398>
01399> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01400> (ha) (cms) (hrs) (mm) (cms)
01401>
01402> TOTAL HYD. 01:EX-1 2.37 .109 1.433 18.147 .000
01403>
01404> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
01405> MINOR SYST 03:toDRAI 2.37 .001 150.250 18.146 .000
01406>
01407> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01408>
01409> Maximum MAJOR SYSTEM storage used = 421. (cu.m.)
01410>
01411> -----
01412> 001:0062
01413> *#-----
01414> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
01415> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
01416> | U.H. Tp (hrs)= .090
01417>
01418> Unit Hyd Qpeak (cms)= .081
01419>
01420> PEAK FLOW (cms)= .023 (i)
01421> TIME TO PEAK (hrs)= 1.100
01422> RUNOFF VOLUME (mm)= 21.201
01423> TOTAL RAINFALL (mm)= 60.373
01424> RUNOFF COEFFICIENT = .351
01425>
01426> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01427>
01428> -----
01429> 001:0063
01430> *#-----
01431> | CALIB STANDHYD | Area (ha)= .45
01432> | 04:EX-3 DT= 1.00 | Total Imp (%)= 65.00 Dir. Conn. (%)= 1.00
01433>
01434> IMPERVIOUS PERVIOUS (i)
01435> Surface Area (ha)= .29 .16
01436> Dep. Storage (mm)= 2.00 8.00
01437> Average Slope (%)= 1.00 20.00
01438> Length (m)= 5.00 25.00
01439> Mannings n = .013 .240
01440>
01441> Max. eff. Inten. (mm/hr)= 220.22 414.90
01442> over (min) 1.00 2.00
01443> Storage Coeff. (min)= .31 (ii) 1.78 (ii)
01444> Unit Hyd. Tpeak (min)= 1.00 2.00
01445> Unit Hyd. peak (cms)= 1.63 .60
01446>
01447> PEAK FLOW (cms)= .00 .16 .159 (iii)
01448> TIME TO PEAK (hrs)= .98 1.00 1.000
01449> RUNOFF VOLUME (mm)= 58.37 35.81 36.038
01450> TOTAL RAINFALL (mm)= 60.37 60.37 60.373
01451> RUNOFF COEFFICIENT = .97 .59 .597
01452>
01453> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01454> CN* = 72.0 Ia = Dep. Storage (Above)
01455> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01456> THAN THE STORAGE COEFFICIENT.
01457> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01458>
01459> -----
01460> 001:0064
01461> *#-----
01462> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
01463> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01464> | U.H. Tp (hrs)= .230
01465>
01466> Unit Hyd Qpeak (cms)= .430
01467>
01468> PEAK FLOW (cms)= .126 (i)
01469> TIME TO PEAK (hrs)= 1.300
01470> RUNOFF VOLUME (mm)= 15.455
01471> TOTAL RAINFALL (mm)= 60.373
01472> RUNOFF COEFFICIENT = .256
01473>
01474> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01475>
01476> -----
01477> 001:0065
01478> *#-----
01479> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01480> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
01481> | Total minor system capacity = .001 (cms)
01482> | Total major system storage [TMJSTO] = 99999. (cu.m.)
01483>
01484> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01485> (ha) (cms) (hrs) (mm) (cms)

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01486> TOTAL HYD. 05:EX-4 2.59 .126 1.300 15.455 .000
01487>
01488> MAJOR SYST 06:BARNAK .00 .000 .000 .000 .000
01489> MINOR SYST 07:toDRAI 2.59 .001 139.883 15.454 .000
01490>
01491> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01492>
01493> Maximum MAJOR SYSTEM storage used = 392. (cu.m.)
01494>
01495> -----
01496> 001:0066
01497> *#-----
01498> | CALIB NASHYD | Area (ha)= 6.64 Curve Number (CN)=72.00
01499> | 05:CL01 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01500> | U.H. Tp (hrs)= .310
01501>
01502> Unit Hyd Qpeak (cms)= .818
01503>
01504> PEAK FLOW (cms)= .317 (i)
01505> TIME TO PEAK (hrs)= 1.417
01506> RUNOFF VOLUME (mm)= 18.147
01507> TOTAL RAINFALL (mm)= 60.373
01508> RUNOFF COEFFICIENT = .301
01509>
01510> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01511>
01512> -----
01513> | ADD HYD (PRE ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01514> (ha) (cms) (hrs) (mm) (cms)
01515>
01516> ID1 01:EX-2 .19 .023 1.10 21.20 .000
01517> +ID2 03:toDRAIN 2.37 .001 150.25 18.15 .000
01518>
01519> +ID3 04:EX-3 .45 .159 1.00 36.04 .000
01520> +ID4 05:CL01 6.64 .317 1.42 18.15 .000
01521> +ID5 07:toDRAIN 2.59 .001 139.88 15.45 .000
01522>
01523> SUM 08:PRE 12.24 .345 1.40 18.28 .000
01524>
01525> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01526>
01527> -----
01528> 001:0067
01529> *#-----
01530> *# PROPOSED CONDITIONS
01531> *#
01532> *#
01533> *#-----
01534> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
01535> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
01536> | U.H. Tp (hrs)= .330
01537>
01538> Unit Hyd Qpeak (cms)= .274
01539>
01540> PEAK FLOW (cms)= .109 (i)
01541> TIME TO PEAK (hrs)= 1.433
01542> RUNOFF VOLUME (mm)= 18.147
01543> TOTAL RAINFALL (mm)= 60.373
01544> RUNOFF COEFFICIENT = .301
01545>
01546> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01547>
01548> -----
01549> 001:0069
01550> *#-----
01551> *# COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01552> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
01553> | Total minor system capacity = .001 (cms)
01554> | Total major system storage [TMJSTO] = 99999. (cu.m.)
01555>
01556> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01557> (ha) (cms) (hrs) (mm) (cms)
01558>
01559> TOTAL HYD. 01:EX-1 2.37 .109 1.433 18.147 .000
01560>
01561> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
01562> MINOR SYST 03:toDRAI 2.37 .001 150.250 18.146 .000
01563>
01564> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01565>
01566> Maximum MAJOR SYSTEM storage used = 421. (cu.m.)
01567>
01568> -----
01569> 001:0070
01570> *#-----
01571> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
01572> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
01573> | U.H. Tp (hrs)= .090
01574>
01575> Unit Hyd Qpeak (cms)= .081
01576>
01577> PEAK FLOW (cms)= .023 (i)
01578> TIME TO PEAK (hrs)= 1.100
01579> RUNOFF VOLUME (mm)= 21.201
01580> TOTAL RAINFALL (mm)= 60.373
01581> RUNOFF COEFFICIENT = .351
01582>
01583> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01584>
01585> -----
01586> 001:0071
01587> *#-----
01588> | CALIB STANDHYD | Area (ha)= .45
01589> | 04:EX-3 DT= 1.00 | Total Imp (%)= 65.00 Dir. Conn. (%)= 1.00
01590>
01591> IMPERVIOUS PERVIOUS (i)
01592> Surface Area (ha)= .29 .16
01593> Dep. Storage (mm)= 2.00 8.00
01594> Average Slope (%)= 1.00 20.00
01595> Length (m)= 5.00 25.00
01596> Mannings n = .013 .240
01597>
01598> Max. eff. Inten. (mm/hr)= 220.22 414.90
01599> over (min) 1.00 2.00
01600> Storage Coeff. (min)= .31 (ii) 1.78 (ii)
01601> Unit Hyd. Tpeak (min)= 1.00 2.00
01602> Unit Hyd. peak (cms)= 1.63 .60
01603>
01604> PEAK FLOW (cms)= .00 .16 .159 (iii)
01605> TIME TO PEAK (hrs)= .98 1.00 1.000
01606> RUNOFF VOLUME (mm)= 58.37 35.81 36.038
01607> TOTAL RAINFALL (mm)= 60.37 60.37 60.373
01608> RUNOFF COEFFICIENT = .97 .59 .597
01609>
01610> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01611> CN* = 72.0 Ia = Dep. Storage (Above)
01612> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01613> THAN THE STORAGE COEFFICIENT.
01614> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01615>
01616> -----
01617> 001:0072
01618> *#-----
01619> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
01620> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00

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01621>----- U.H. Tp(hrs)= .230
01622>
01623> Unit Hyd Qpeak (cms)= .430
01624>
01625> PEAK FLOW (cms)= .126 (i)
01626> TIME TO PEAK (hrs)= 1.300
01627> RUNOFF VOLUME (mm)= 15.455
01628> TOTAL RAINFALL (mm)= 60.373
01629> RUNOFF COEFFICIENT = .256
01630>
01631> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01632>-----
01633>-----
01634> 001:0073-----
01635>-----
01636> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01637> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
01638> | | Total minor system capacity = .001 (cms)
01639> | | Total major system storage [TMJSTO] = 99999.(cu.m.)
01640>-----
01641> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01642> (ha) (cms) (hrs) (mm) (cms)
01643> TOTAL HYD. 05:EX-4 2.59 .126 1.300 15.455 .000
01644>-----
01645> MAJOR SYST 06:BANMAN .00 .000 .000 .000 .000
01646> MINOR SYST 07:toDRAI 2.59 .001 139.883 15.454 .000
01647>
01648> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01649>
01650> Maximum MAJOR SYSTEM storage used = 392.(cu.m.)
01651>-----
01652>-----
01653> 001:0074-----
01654>-----
01655> CALIB STANDHYD | Area (ha)= 6.64
01656> | 05:C201 DT= 1.00 | Total Imp(%)= 90.00 Dir. Conn.(%)= 80.00
01657>-----
01658>-----
01659> Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
01660> Dep. Storage (mm)= 2.00 5.00
01661> Average Slope (%)= 1.00 2.00
01662> Length (m)= 50.00 25.00
01663> Mannings n = .013 .250
01664>-----
01665> Max.eff.Inten.(mm/hr)= 220.22 144.22
01666> over (min) 1.00 6.00
01667> Storage Coeff. (min)= 1.23 (ii) 5.83 (ii)
01668> Unit Hyd. Tpeak (min)= 1.00 6.00
01669> Unit Hyd. peak (cms)= .95 .19
01670>-----
01671> PEAK FLOW (cms)= 3.21 .18 *TOTALS*
01672> TIME TO PEAK (hrs)= 1.00 1.08 3.306 (iii)
01673> RUNOFF VOLUME (mm)= 58.37 24.08 1.10 21.20 .000
01674> TOTAL RAINFALL (mm)= 60.37 60.37 60.373
01675> RUNOFF COEFFICIENT = .97 .40 .853
01676>
01677> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01678> CN* = 61.0 Ia = Dep. Storage (Above)
01679> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01680> THAN THE STORAGE COEFFICIENT.
01681> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01682>-----
01683>-----
01684> 001:0075-----
01685>-----
01686> ADD HYD (POST ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01687> (ha) (cms) (hrs) (mm) (cms)
01688> +ID1 01:EX-2 .19 .023 1.10 21.20 .000
01689> +ID2 03:toDRAIN 2.37 .001 150.25 18.15 .000
01690> +ID3 04:EX-3 .45 .159 1.00 36.04 .000
01691> +ID4 05:C201 6.64 3.306 1.00 51.52 .000
01692> +IDS 07:toDRAIN 2.59 .001 139.88 15.45 .000
01693>-----
01694> SUM 09:POST 12.24 3.478 1.00 36.37 .000
01695>-----
01696> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01697>-----
01698>-----
01699> 001:0076-----
01700>-----
01701> ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01702> | IN>09:(POST ) |
01703> | OUT<01:(SWMF ) |
01704>-----
01705> OUTFLOW STORAGE | OUTFLOW STORAGE
01706> (cms) (ha.m.) | (cms) (ha.m.)
01707> .000 .0000E+00 | .079 .1497E+00
01708> .059 .1884E-01 | .080 .1683E+00
01709> .069 .3388E-01 | .082 .1875E+00
01710> .070 .4889E-01 | .083 .2073E+00
01711> .072 .6439E-01 | .085 .2276E+00
01712> .073 .8041E-01 | .086 .2485E+00
01713> .075 .9693E-01 | .087 .2700E+00
01714> .076 .1140E+00 | .299 .4003E+00
01715> .078 .1316E+00 | .000 .0000E+00
01716>-----
01717> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01718> (ha) (cms) (hrs) (mm)
01719> INFLOW<09:(POST ) 12.24 3.478 1.000 36.373
01720> OUTFLOW<01:(SWMF ) 12.24 .112 2.317 36.375
01721> OVERFLOW<02:(000003) .00 .000 .000 .000
01722>-----
01723> TOTAL NUMBER OF SIMULATED OVERFLOWS = 0
01724> CUMULATIVE TIME OF OVERFLOWS (hours)= .00
01725> PERCENTAGE OF TIME OVERFLOWING (%)= .00
01726>-----
01727> PEAK FLOW REDUCTION [Qout/Qin] (%)= 3.209
01728> TIME SHIFT OF PEAK FLOW (min)= 79.00
01729> MAXIMUM STORAGE USED (ha.m.)=.2851E+00
01730>-----
01731>-----
01732> 001:0077-----
01733>-----
01734> DIVERT HYD |
01735> | INID=01 (SWMF ) |
01736>-----
01737> Outflow / Inflow Relationships
01738> Flow 03 + Flow 04 + Flow 05 = Total
01739> (cms) (cms) (cms) (cms)
01740> .000 .000 .000 .000
01741> .000 .059 .000 .059
01742> .007 .061 .000 .069
01743> .007 .063 .000 .070
01744> .008 .064 .000 .072
01745> .008 .065 .000 .073
01746> .008 .066 .000 .075
01747> .009 .068 .000 .076
01748> .009 .069 .000 .078
01749> .009 .070 .000 .079
01750> .009 .071 .000 .080
01751> .010 .072 .000 .082
01752> .010 .073 .000 .083
01753> .010 .074 .000 .085
01754> .010 .075 .000 .086
01755> .011 .077 .000 .087

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01756> .011 .078 .210 .299
01757>-----
01758> NHYD AREA QPEAK TpeakDate_hh:mm R.V. NFE WetHrs
01759> (ha) (cms) (hrs) (mm) (hrs)
01760> IDin = 01:SWMF 12.24 .112 No date 2:19 36.375 1 158.
01761>-----
01762> IDout= 03:Infillt 1.04 .011 No date 2:19 36.375 1 12.
01763> IDout= 04:Driflic 10.88 .077 No date 2:19 36.375 1 158.
01764> IDout= 05:Emerge .21 .024 No date 2:19 36.375 1 2.
01765>-----
01766> 001:0078-----
01767> **-----
01768> **-----
01769> ** 50-year
01770> **-----
01771> **-----
01772> **-----
01773>-----
01774> CHICAGO STORM | IDF curve parameters: A=1499.060
01775> | Ptotal= 66.11 mm | B= 4.188
01776> | | C= .809
01777> used in: INTENSITY = A / (t + B)^C
01778>-----
01779> Duration of storm = 3.00 hrs
01780> Storm time step = 5.00 min
01781> Time to peak ratio = .33
01782>-----
01783> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01784> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01785> .08 5.048 | .83 27.983 | 1.58 14.045 | 2.33 6.561
01786> .17 5.501 | .92 68.751 | 1.67 12.381 | 2.42 6.216
01787> .25 6.054 | 1.00 249.214 | 1.75 11.088 | 2.50 5.909
01788> .33 6.747 | 1.08 90.168 | 1.83 10.056 | 2.58 5.633
01789> .42 7.643 | 1.17 47.427 | 1.92 9.210 | 2.67 5.384
01790> .50 8.846 | 1.25 31.940 | 2.00 8.506 | 2.75 5.158
01791> .58 10.558 | 1.33 24.096 | 2.08 7.909 | 2.83 4.952
01792> .67 13.195 | 1.42 19.393 | 2.17 7.396 | 2.92 4.763
01793> .75 17.808 | 1.50 16.269 | 2.25 6.951 | 3.00 4.590
01794>-----
01795>-----
01796> 001:0079-----
01797> **-----
01798> **-----
01799> ** EXISTING CONDITIONS
01800> **-----
01801> **-----
01802>-----
01803> CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
01804> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
01805> U.H. Tp(hrs)= .330
01806>-----
01807> Unit Hyd Qpeak (cms)= .274
01808>-----
01809> PEAK FLOW (cms)= .130 (i)
01810> TIME TO PEAK (hrs)= 1.433
01811> RUNOFF VOLUME (mm)= 21.525
01812> TOTAL RAINFALL (mm)= 66.112
01813> RUNOFF COEFFICIENT = .326
01814>
01815> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01816>-----
01817>-----
01818>-----
01819>-----
01820> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01821> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
01822> | | Total minor system capacity = .001 (cms)
01823> | | Total major system storage [TMJSTO] = 99999.(cu.m.)
01824>-----
01825> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01826> (ha) (cms) (hrs) (mm) (cms)
01827> TOTAL HYD. 01:EX-1 2.37 .130 1.433 21.525 .000
01828>-----
01829> MAJOR SYST 02:THAMES .00 .000 .000 .000 .000
01830> MINOR SYST 03:toDRAI 2.37 .001 178.033 21.524 .000
01831>-----
01832> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01833>-----
01834> Maximum MAJOR SYSTEM storage used = 500.(cu.m.)
01835>-----
01836>-----
01837> 001:0081-----
01838>-----
01839> CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
01840> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01841> U.H. Tp(hrs)= .090
01842>-----
01843> Unit Hyd Qpeak (cms)= .081
01844>-----
01845> PEAK FLOW (cms)= .027 (i)
01846> TIME TO PEAK (hrs)= 1.083
01847> RUNOFF VOLUME (mm)= 24.839
01848> TOTAL RAINFALL (mm)= 66.112
01849> RUNOFF COEFFICIENT = .376
01850>-----
01851> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01852>-----
01853>-----
01854> 001:0082-----
01855>-----
01856> CALIB STANDHYD | Area (ha)= .45
01857> | 04:EX-3 DT= 1.00 | Total Imp(%)= 65.00 Dir. Conn.(%)= 1.00
01858>-----
01859> IMPERVIOUS PERVIOUS (i)
01860> Surface Area (ha)= .29 .16
01861> Dep. Storage (mm)= 2.00 8.00
01862> Average Slope (%)= 1.00 20.00
01863> Length (m)= 5.00 25.00
01864> Mannings n = .013 .240
01865>-----
01866> Max.eff.Inten.(mm/hr)= 249.21 491.16
01867> over (min) 1.00 2.00
01868> Storage Coeff. (min)= .29 (ii) 1.67 (ii)
01869> Unit Hyd. Tpeak (min)= 1.00 2.00
01870> Unit Hyd. peak (cms)= 1.64 .62
01871>-----
01872> PEAK FLOW (cms)= .00 .19 .191 (iii)
01873> TIME TO PEAK (hrs)= 1.00 1.00 1.000
01874> RUNOFF VOLUME (mm)= 64.11 40.78 41.014
01875> TOTAL RAINFALL (mm)= 66.11 66.11 66.112
01876> RUNOFF COEFFICIENT = .97 .62 .620
01877>-----
01878> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01879> CN* = 72.0 Ia = Dep. Storage (Above)
01880> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01881> THAN THE STORAGE COEFFICIENT.
01882> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01883>-----
01884>-----
01885> 001:0083-----
01886>-----
01887> CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
01888> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
01889> U.H. Tp(hrs)= .230
01890>-----

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01891> Unit Hyd Qpeak (cms)= .430
 01892> PEAK FLOW (cms)= .151 (i)
 01894> TIME TO PEAK (hrs)= 1.300
 01895> RUNOFF VOLUME (mm)= 18.432
 01896> TOTAL RAINFALL (mm)= 66.112
 01897> RUNOFF COEFFICIENT = .279
 01899> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01900>-----
 01901>-----
 01902> 001:0084
 01903>-----
 01904> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
 01905> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
 01906> | Total minor system capacity | = .001 (cms)
 01907> | Total major system storage [TMJSTO] = 99999. (cu.m.)
 01908>-----
 01909> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01910> (ha) (cms) (hrs) (mm) (cms)
 01911> TOTAL HYD. 05:EX-4 2.59 .151 1.300 18.432 .000
 01912>-----
 01913> | MINOR SYST 06:RAINMAN | .001 .000 .000 .000
 01914> MINOR SYST 07:toDRAIN 2.59 .001 166.633 18.430 .000
 01915>-----
 01916> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01917> Maximum MAJOR SYSTEM storage used = 469. (cu.m.)
 01918>-----
 01919>-----
 01920>-----
 01921> 001:0085
 01922>-----
 01923> | CALIB NASHHYD | Area (ha)= 6.64 Curve Number (CN)=72.00
 01924> | 05:C101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
 01925>-----
 01926> U.H. Tp(hrs)= .310
 01927>-----
 01928> Unit Hyd Qpeak (cms)= .818
 01929> PEAK FLOW (cms)= .379 (i)
 01930> TIME TO PEAK (hrs)= 1.400
 01931> RUNOFF VOLUME (mm)= 21.525
 01932> TOTAL RAINFALL (mm)= 66.112
 01933> RUNOFF COEFFICIENT = .326
 01934>-----
 01935> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01936>-----
 01937>-----
 01938> 00086
 01939>-----
 01940> | ADD HYD (PRE) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01941> (ha) (cms) (hrs) (mm) (cms)
 01942> ID1 01:EX-2 .19 .027 1.08 24.84 .000
 01943> +ID3 04:EX-3 2.37 .001 178.03 21.52 .000
 01944> +ID4 05:C101 6.64 .379 1.40 21.52 .000
 01945> +ID5 07:toDRAIN 2.59 .001 166.63 18.43 .000
 01946>-----
 01947> SUM 08:PRE 12.24 .410 1.38 21.64 .000
 01948>-----
 01949>-----
 01950> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01951>-----
 01952>-----
 01953> 00087
 01954>-----
 01955> | PRINT HYD | AREA (ha)= 12.240
 01956> | ID=08 (PRE) | QPEAK (cms)= .410 (i)
 01957> | DT= 1.00 FCYCM= 5 | TPEAK (hrs)= 1.383
 01958> | VOLUME (mm)= 21.640
 01959>-----
 01960> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01961> TIME FLOW | TIME FLOW | TIME FLOW | TIME FLOW
 01962> hrs cms | hrs cms | hrs cms | hrs cms
 01963> .00 .000 | .35 67.00 | 71.42 .002 | 107.01 .002
 01964> .08 .000 | .35 75.00 | 71.42 .002 | 107.09 .002
 01965> .17 .000 | .35 83.00 | 71.50 .002 | 107.18 .002
 01966> .25 .000 | .35 91.00 | 71.58 .002 | 107.26 .002
 01967> .33 .000 | .36 00.00 | 71.67 .002 | 107.34 .002
 01968> .42 .000 | .36 08.00 | 71.75 .002 | 107.43 .002
 01969> .50 .000 | .36 17.00 | 71.83 .002 | 107.51 .002
 01970> .58 .001 | .36 25.00 | 71.92 .002 | 107.59 .002
 01971> .67 .002 | .36 33.00 | 72.00 .002 | 107.68 .002
 01972> .75 .004 | .36 41.00 | 72.08 .002 | 107.76 .002
 01973> .83 .009 | .36 49.00 | 72.17 .002 | 107.84 .002
 01974> .92 .037 | .36 58.00 | 72.25 .002 | 107.93 .002
 01975> 1.00 .238 | .36 67.00 | 72.33 .002 | 108.01 .002
 01976> 1.08 .249 | .36 75.00 | 72.42 .002 | 108.09 .002
 01977> 1.17 .312 | .36 83.00 | 72.50 .002 | 108.18 .002
 01978> 1.25 .374 | .36 91.00 | 72.59 .002 | 108.26 .002
 01979> 1.33 .406 | .36 99.00 | 72.67 .002 | 108.34 .002
 01980> 1.42 .408 | .37 08.00 | 72.75 .002 | 108.43 .002
 01981> 1.50 .388 | .37 17.00 | 72.83 .002 | 108.51 .002
 01982> 1.58 .355 | .37 25.00 | 72.92 .002 | 108.59 .002
 01983> 1.67 .318 | .37 33.00 | 73.01 .002 | 108.68 .002
 01984> 1.75 .280 | .37 42.00 | 73.08 .002 | 108.76 .002
 01985> 1.83 .245 | .37 50.00 | 73.17 .002 | 108.84 .002
 01986> 1.92 .215 | .37 58.00 | 73.25 .002 | 108.93 .002
 01987> 2.00 .188 | .37 67.00 | 73.33 .002 | 109.01 .002
 01988> 2.08 .166 | .37 75.00 | 73.42 .002 | 109.09 .002
 01989> 2.17 .148 | .37 83.00 | 73.50 .002 | 109.18 .002
 01990> 2.25 .133 | .37 91.00 | 73.58 .002 | 109.26 .002
 01991> 2.33 .121 | .38 00.00 | 73.67 .002 | 109.34 .002
 01992> 2.42 .111 | .38 08.00 | 73.75 .002 | 109.43 .002
 01993> 2.50 .102 | .38 17.00 | 73.83 .002 | 109.51 .002
 01994> 2.58 .095 | .38 25.00 | 73.92 .002 | 109.59 .002
 01995> 2.67 .089 | .38 33.00 | 74.00 .002 | 109.68 .002
 01996> 2.75 .083 | .38 41.00 | 74.08 .002 | 109.76 .002
 01997> 2.83 .079 | .38 50.00 | 74.17 .002 | 109.84 .002
 01998> 2.92 .075 | .38 58.00 | 74.25 .002 | 109.93 .002
 01999> 3.00 .071 | .38 67.00 | 74.33 .002 | 110.01 .002
 02000> 3.08 .062 | .38 75.00 | 74.42 .002 | 110.09 .002
 02001> 3.17 .055 | .38 83.00 | 74.50 .002 | 110.18 .002
 02002> 3.25 .046 | .38 91.00 | 74.58 .002 | 110.26 .002
 02003> 3.33 .037 | .39 00.00 | 74.67 .002 | 110.34 .002
 02004> 3.42 .029 | .39 08.00 | 74.75 .002 | 110.43 .002
 02005> 3.50 .022 | .39 17.00 | 74.83 .002 | 110.51 .002
 02006> 3.58 .016 | .39 25.00 | 74.92 .002 | 110.59 .002
 02007> 3.67 .012 | .39 33.00 | 75.00 .002 | 110.68 .002
 02008> 3.75 .009 | .39 41.00 | 75.08 .002 | 110.76 .002
 02009> 3.83 .007 | .39 50.00 | 75.17 .002 | 110.84 .002
 02010> 3.92 .005 | .39 58.00 | 75.25 .002 | 110.93 .002
 02011> 4.00 .004 | .39 67.00 | 75.33 .002 | 111.01 .002
 02012> 4.08 .003 | .39 75.00 | 75.42 .002 | 111.09 .002
 02013> 4.17 .002 | .39 83.00 | 75.50 .002 | 111.18 .002
 02014> 4.25 .002 | .39 91.00 | 75.58 .002 | 111.26 .002
 02015> 4.33 .002 | .40 00.00 | 75.67 .002 | 111.34 .002
 02016> 4.42 .002 | .40 08.00 | 75.75 .002 | 111.43 .002
 02017> 4.50 .002 | .40 17.00 | 75.84 .002 | 111.51 .002
 02018> 4.58 .002 | .40 25.00 | 75.92 .002 | 111.59 .002
 02019> 4.67 .002 | .40 33.00 | 76.00 .002 | 111.68 .002
 02020> 4.75 .002 | .40 41.00 | 76.09 .002 | 111.76 .002
 02021> 4.83 .002 | .40 50.00 | 76.17 .002 | 111.84 .002
 02022> 4.92 .002 | .40 58.00 | 76.25 .002 | 111.93 .002
 02023> 5.00 .002 | .40 67.00 | 76.34 .002 | 112.01 .002
 02024> 5.08 .002 | .40 75.00 | 76.42 .002 | 112.09 .002
 02025> 5.17 .002 | .40 83.00 | 76.50 .002 | 112.18 .002

02026> 5.25 .002 | 40.92 .002 | 76.59 .002 | 112.26 .002 | 147.92 .002
 02027> 5.33 .002 | 41.00 .002 | 76.67 .002 | 112.34 .002 | 148.01 .002
 02028> 5.41 .002 | 41.08 .002 | 76.75 .002 | 112.42 .002 | 148.10 .002
 02029> 5.50 .002 | 41.17 .002 | 76.84 .002 | 112.51 .002 | 148.17 .002
 02030> 5.58 .002 | 41.25 .002 | 76.92 .002 | 112.59 .002 | 148.26 .002
 02031> 5.67 .002 | 41.33 .002 | 77.00 .002 | 112.68 .002 | 148.34 .002
 02032> 5.75 .002 | 41.42 .002 | 77.09 .002 | 112.76 .002 | 148.42 .002
 02033> 5.83 .002 | 41.50 .002 | 77.17 .002 | 112.84 .002 | 148.51 .002
 02034> 5.92 .002 | 41.58 .002 | 77.25 .002 | 112.93 .002 | 148.59 .002
 02035> 6.00 .002 | 41.67 .002 | 77.34 .002 | 113.01 .002 | 148.67 .002
 02036> 6.08 .002 | 41.75 .002 | 77.42 .002 | 113.09 .002 | 148.76 .002
 02037> 6.17 .002 | 41.83 .002 | 77.50 .002 | 113.18 .002 | 148.84 .002
 02038> 6.25 .002 | 41.91 .002 | 77.58 .002 | 113.26 .002 | 148.92 .002
 02039> 6.33 .002 | 42.00 .002 | 77.67 .002 | 113.34 .002 | 149.01 .002
 02040> 6.42 .002 | 42.08 .002 | 77.75 .002 | 113.43 .002 | 149.09 .002
 02041> 6.50 .002 | 42.17 .002 | 77.84 .002 | 113.51 .002 | 149.17 .002
 02042> 6.58 .002 | 42.25 .002 | 77.92 .002 | 113.59 .002 | 149.26 .002
 02043> 6.67 .002 | 42.33 .002 | 78.00 .002 | 113.68 .002 | 149.34 .002
 02044> 6.75 .002 | 42.42 .002 | 78.09 .002 | 113.76 .002 | 149.42 .002
 02045> 6.83 .002 | 42.50 .002 | 78.17 .002 | 113.84 .002 | 149.51 .002
 02046> 6.92 .002 | 42.58 .002 | 78.25 .002 | 113.93 .002 | 149.59 .002
 02047> 7.00 .002 | 42.67 .002 | 78.34 .002 | 114.01 .002 | 149.67 .002
 02048> 7.08 .002 | 42.75 .002 | 78.42 .002 | 114.09 .002 | 149.76 .002
 02049> 7.17 .002 | 42.83 .002 | 78.50 .002 | 114.18 .002 | 149.84 .002
 02050> 7.25 .002 | 42.92 .002 | 78.59 .002 | 114.26 .002 | 149.92 .002
 02051> 7.33 .002 | 43.00 .002 | 78.67 .002 | 114.34 .002 | 150.01 .002
 02052> 7.42 .002 | 43.08 .002 | 78.75 .002 | 114.43 .002 | 150.09 .002
 02053> 7.50 .002 | 43.17 .002 | 78.84 .002 | 114.51 .002 | 150.17 .002
 02054> 7.58 .002 | 43.25 .002 | 78.92 .002 | 114.59 .002 | 150.26 .002
 02055> 7.67 .002 | 43.33 .002 | 79.00 .002 | 114.68 .002 | 150.34 .002
 02056> 7.75 .002 | 43.42 .002 | 79.09 .002 | 114.76 .002 | 150.42 .002
 02057> 7.83 .002 | 43.50 .002 | 79.17 .002 | 114.84 .002 | 150.51 .002
 02058> 7.92 .002 | 43.59 .002 | 79.25 .002 | 114.93 .002 | 150.59 .002
 02059> 8.00 .002 | 43.67 .002 | 79.34 .002 | 115.01 .002 | 150.67 .002
 02060> 8.08 .002 | 43.75 .002 | 79.42 .002 | 115.09 .002 | 150.76 .002
 02061> 8.17 .002 | 43.83 .002 | 79.50 .002 | 115.18 .002 | 150.84 .002
 02062> 8.25 .002 | 43.92 .002 | 79.59 .002 | 115.26 .002 | 150.92 .002
 02063> 8.34 .002 | 44.00 .002 | 79.67 .002 | 115.34 .002 | 151.01 .002
 02064> 8.42 .002 | 44.08 .002 | 79.75 .002 | 115.43 .002 | 151.09 .002
 02065> 8.50 .002 | 44.17 .002 | 79.84 .002 | 115.51 .002 | 151.17 .002
 02066> 8.58 .002 | 44.25 .002 | 79.92 .002 | 115.59 .002 | 151.26 .002
 02067> 8.67 .002 | 44.33 .002 | 80.00 .002 | 115.68 .002 | 151.34 .002
 02068> 8.75 .002 | 44.42 .002 | 80.08 .002 | 115.76 .002 | 151.42 .002
 02069> 8.83 .002 | 44.50 .002 | 80.17 .002 | 115.84 .002 | 151.51 .002
 02070> 8.92 .002 | 44.58 .002 | 80.25 .002 | 115.93 .002 | 151.59 .002
 02071> 9.00 .002 | 44.67 .002 | 80.34 .002 | 116.01 .002 | 151.67 .002
 02072> 9.08 .002 | 44.75 .002 | 80.42 .002 | 116.09 .002 | 151.76 .002
 02073> 9.17 .002 | 44.83 .002 | 80.50 .002 | 116.18 .002 | 151.84 .002
 02074> 9.25 .002 | 44.92 .002 | 80.59 .002 | 116.26 .002 | 151.92 .002
 02075> 9.33 .002 | 45.00 .002 | 80.67 .002 | 116.34 .002 | 152.01 .002
 02076> 9.42 .002 | 45.08 .002 | 80.75 .002 | 116.43 .002 | 152.09 .002
 02077> 9.50 .002 | 45.17 .002 | 80.84 .002 | 116.51 .002 | 152.17 .002
 02078> 9.59 .002 | 45.25 .002 | 80.92 .002 | 116.59 .002 | 152.26 .002
 02079> 9.67 .002 | 45.33 .002 | 81.00 .002 | 116.68 .002 | 152.34 .002
 02080> 9.75 .002 | 45.42 .002 | 81.09 .002 | 116.76 .002 | 152.42 .002
 02081> 9.83 .002 | 45.50 .002 | 81.17 .002 | 116.84 .002 | 152.51 .002
 02082> 9.92 .002 | 45.58 .002 | 81.25 .002 | 116.93 .002 | 152.59 .002
 02083> 10.00 .002 | 45.67 .002 | 81.34 .002 | 117.01 .002 | 152.67 .002
 02084> 10.08 .002 | 45.75 .002 | 81.42 .002 | 117.09 .002 | 152.76 .002
 02085> 10.17 .002 | 45.83 .002 | 81.50 .002 | 117.18 .002 | 152.84 .002
 02086> 10.25 .002 | 45.92 .002 | 81.59 .002 | 117.26 .002 | 152.92 .002
 02087> 10.33 .002 | 46.00 .002 | 81.67 .002 | 117.34 .002 | 153.01 .002
 02088> 10.42 .002 | 46.08 .002 | 81.75 .002 | 117.43 .002 | 153.09 .002
 02089> 10.50 .002 | 46.17 .002 | 81.84 .002 | 117.51 .002 | 153.17 .002
 02090> 10.58 .002 | 46.25 .002 | 81.92 .002 | 117.59 .002 | 153.26 .002
 02091> 10.67 .002 | 46.33 .002 | 82.00 .002 | 117.68 .002 | 153.34 .002
 02092> 10.75 .002 | 46.42 .002 | 82.09 .002 | 117.76 .002 | 153.42 .002
 02093> 10.83 .002 | 46.50 .002 | 82.17 .002 | 117.84 .002 | 153.51 .002
 02094> 10.92 .002 | 46.58 .002 | 82.25 .002 | 117.93 .002 | 153.59 .002
 02095> 11.00 .002 | 46.67 .002 | 82.34 .002 | 118.01 .002 | 153.67 .002
 02096> 11.08 .002 | 46.75 .002 | 82.42 .002 | 118.09 .002 | 153.76 .002
 02097> 11.17 .002 | 46.83 .002 | 82.50 .002 | 118.18 .002 | 153.84 .002
 02098> 11.25 .002 | 46.92 .002 | 82.59 .002 | 118.26 .002 | 153.92 .002
 02099> 11.33 .002 | 47.00 .002 | 82.67 .002 | 118.34 .002 | 154.01 .002
 02100> 11.42 .002 | 47.08 .002 | 82.75 .002 | 118.43 .002 | 154.09 .002
 02101> 11.50 .002 | 47.17 .002 | 82.84 .002 | 118.51 .002 | 154.17 .002
 02102> 11.58 .002 | 47.25 .002 | 82.92 .002 | 118.59 .002 | 154.26 .002
 02103> 11.67 .002 | 47.33 .002 | 83.00 .002 | 118.68 .002 | 154.34 .002
 02104> 11.75 .002 | 47.42 .002 | 83.09 .002 | 118.76 .002 | 154.42 .002
 02105> 11.83 .002 | 47.50 .002 | 83.17 .002 | 118.84 .002 | 154.51 .002
 02106> 11.92 .002 | 47.58 .002 | 83.25 .002 | 118.93 .002 | 154.59 .002
 02107> 12.00 .002 | 47.67 .002 | 83.34 .002 | 119.01 .002 | 154.67 .002
 02108> 12.09 .002 | 47.75 .002 | 83.42 .002 | 119.09 .002 | 154.76 .002
 02109> 12.17 .002 | 47.83 .002 | 83.50 .002 | 119.18 .002 | 154.84 .002
 02110> 12.25 .002 | 47.92 .002 | 83.59 .002 | 119.26 .002 | 154.92 .002
 02111> 12.33 .002 | 48.00 .002 | 83.67 .002 | 119.34 .002 | 155.01 .002
 02112> 12.42 .002 | 48.08 .002 | 83.75 .002 | 119.43 .002 | 155.09 .002
 02113> 12.50 .002 | 48.17 .002 | 83.84 .002 | 119.51 .002 | 155.17 .002
 02114> 12.58 .002 | 48.25 .002 | 83.92 .002 | 119.59 .002 | 155.26 .002
 02115> 12.67 .002 | 48.33 .002 | 84.00 .002 | 119.68 .002 | 155.34 .002
 02116> 12.75 .002 | 48.42 .002 | 84.09 .002 | 119.76 .002 | 155.42 .002
 02117> 12.83 .002 | 48.50 .002 | 84.17 .002 | 119.84 .002 | 155.51 .002
 02118> 12.92 .002 | 48.59 .002 | 84.25 .002 | 119.93 .002 | 155.59 .002
 02119> 13.00 .002 | 48.67 .002 | 84.34 .002 | 120.01 .002 | 155.67 .002
 02120> 13.08 .002 | 48.75 .002 | 84.42 .002 | 120.09 .002 | 155.76 .002
 02121> 13.17 .002 | 48.83 .002 | 84.50 .002 | 120.18 .002 | 155.84 .002
 02122> 13.25 .002 | 48.92 .002 | 84.59 .002 | 120.26 .002 | 155.92 .002
 02123> 13.33 .002 | 49.00 .002 | 84.67 .002 | 120.34 .002 | 156.01 .002
 02124> 13.42 .002 | 49.08 .002 | 84.75 .002 | 120.43 .002 | 156.09 .002
 02125> 13.50 .002 | 49.17 .002 | 84.84 .002 | 120.51 .002 | 156.17 .002
 02126> 13.58 .002 | 49.25 .002 | 84.92 .002 | 120.59 .002 | 156.26 .002
 02127> 13.67 .002 | 49.33 .002 | 85.00 .002 | 120.68 .002 | 156.34 .002
 02128> 13.75 .002 | 49.42 .002 | 85.09 .002 | 120.76 .002 | 156.42 .002
 02129> 13.83 .002 | 49.50 .002 | 85.17 .002 | 120.84 .002 | 156.51 .002
 02130> 13.92 .002 | 49.58 .002 | 85.25 .002 | 120.93 .002 | 156.59 .002

Table with 5 columns: ID, Value, Unit, Description, Value, Unit, Description. Contains data for various hydrologic model parameters and results, including flow rates and storage volumes.

Table with 5 columns: ID, Value, Unit, Description, Value, Unit, Description. Contains data for various hydrologic model parameters and results, including flow rates and storage volumes.

02431>-----
02432> 001:0090-----
02433>-----
02434>-----
02435> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
02436> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02437>-----
02438>-----
02439> Unit Hyd Qpeak (cms)= .081
02440>-----
02441> PEAK FLOW (cms)= .027 (i)
02442> TIME TO PEAK (hrs)= 1.083
02443> RUNOFF VOLUME (mm)= 24.839
02444> TOTAL RAINFALL (mm)= 66.112
02445> RUNOFF COEFFICIENT = .376
02446>-----
02447> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02448>-----
02449>-----
02450> 001:0091-----
02451>-----
02452> | CALIB STANDHYD | Area (ha)= .45
02453> | 04:EX-3 DT= 1.00 | Total Imp(%)= 65.00 Dir. Conn.(%)= 1.00
02454>-----
02455>-----
02456> IMPERVIOUS PERVIOUS (i)
02457> Surface Area (ha)= .29 .16
02458> Dep. Storage (mm)= 2.00 8.00
02459> Average Slope (%)= 1.00 20.00
02460> Length (m)= 5.00 25.00
02461> Mannings n = .013 .240
02462>-----
02463> Max.eff.Inten.(mm/hr)= 249.21 491.16
02464> over (min) 1.00 2.00
02465> Storage Coeff. (min)= .29 (ii) 1.67 (ii)
02466> Unit Hyd. Tpeak (min)= 1.00 2.00
02467> Unit Hyd. peak (cms)= 1.64 .62
02468>-----
02469> PEAK FLOW (cms)= .00 .19 *TOTALS*
02470> TIME TO PEAK (hrs)= .98 1.00 1.000 (iii)
02471> RUNOFF VOLUME (mm)= 64.11 40.78 41.014
02472> TOTAL RAINFALL (mm)= 66.11 66.11 66.112
02473> RUNOFF COEFFICIENT = .97 .62 .620
02474>-----
02475> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02476> CN* = 72.0 Ia = Dep. Storage (Above)
02477> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02478> THAN THE STORAGE COEFFICIENT.
02479> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02480>-----
02481> 001:0092-----
02482>-----
02483> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
02484> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
02485>-----
02486>-----
02487> U.H. Tp(hrs)= .230
02488>-----
02489> Unit Hyd Qpeak (cms)= .430
02490>-----
02491> PEAK FLOW (cms)= .151 (i)
02492> TIME TO PEAK (hrs)= 1.300
02493> RUNOFF VOLUME (mm)= 18.432
02494> TOTAL RAINFALL (mm)= 66.112
02495> RUNOFF COEFFICIENT = .279
02496>-----
02497> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02498>-----
02499>-----
02500> 001:0093-----
02501>-----
02502> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
02503> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
02504>-----
02505> Total minor system capacity = .001 (cms)
02506> Total major system storage [TMJSTO] = 99999.(cu.m.)
02507>-----
02508> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02509> (ha) (cms) (hrs) (mm) (cms)
02510> TOTAL HYD. 05:EX-4 2.59 .151 1.300 18.432 .000
02511>-----
02512> MAJOR SYST 06:BNMAN .00 .000 .000 .000 .000
02513> MINOR SYST 07:toDRAI 2.59 .001 166.633 18.430 .000
02514>-----
02515> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02516>-----
02517> Maximum MAJOR SYSTEM storage used = 469.(cu.m.)
02518>-----
02519> 001:0094-----
02520>-----
02521> | CALIB STANDHYD | Area (ha)= 6.64
02522> | 05:C201 DT= 1.00 | Total Imp(%)= 90.00 Dir. Conn.(%)= 80.00
02523>-----
02524> IMPERVIOUS PERVIOUS (i)
02525> Surface Area (ha)= 5.98 .66
02526> Dep. Storage (mm)= 2.00 5.00
02527> Average Slope (%)= 1.00 2.00
02528> Length (m)= 50.00 25.00
02529> Mannings n = .013 .250
02530>-----
02531> Max.eff.Inten.(mm/hr)= 249.21 191.11
02532> over (min) 1.00 5.00
02533> Storage Coeff. (min)= 1.17 (ii) 5.28 (ii)
02534> Unit Hyd. Tpeak (min)= 1.00 5.00
02535> Unit Hyd. peak (cms)= .98 .22
02536>-----
02537> PEAK FLOW (cms)= 3.64 .22 *TOTALS*
02538> TIME TO PEAK (hrs)= 1.00 1.07 1.000 (iii)
02539> RUNOFF VOLUME (mm)= 64.11 27.94 56.879
02540> TOTAL RAINFALL (mm)= 66.11 66.11 66.112
02541> RUNOFF COEFFICIENT = .97 .42 .860
02542>-----
02543> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02544> CN* = 61.0 Ia = Dep. Storage (Above)
02545> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02546> THAN THE STORAGE COEFFICIENT.
02547> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02548>-----
02549>-----
02550> 001:0095-----
02551>-----
02552> | ADD HYD (POST) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02553> (ha) (cms) (hrs) (mm) (cms)
02554> ID1 01:EX-2 .19 .027 1.08 24.84 .000
02555> +ID3 04:EX-3 2.37 .001 178.03 21.52 .000
02556> +ID4 05:C201 6.64 3.786 1.00 56.88 .000
02557> +IDS 07:toDRAIN 2.59 .001 166.63 18.43 .000
02558>-----
02559> SUM 09:POST 12.24 3.994 1.00 40.80 .000
02560>-----
02561> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02562>-----
02563> 001:0096-----
02564>-----
02565> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.

02566> | IN>09:(POST) |
02567> | OUT<01:(SWMF) |
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Table with 14 columns of numerical data representing hydrologic model results. The first column contains alphanumeric codes (e.g., 02971>), and the subsequent columns contain numerical values ranging from approximately 10.25 to 32.67. The data is organized into two main vertical sections.

Table with columns for ID, NHYD, AREA, QPEAK, TPEAK, R.V., DWF. Contains data for various sub-catchments and a summary section for PROPOSED CONDITIONS.

Table with columns for ID, AREA, QPEAK, TPEAK, R.V., DWF. Contains detailed hydrologic data for sub-catchments 03376 through 03510, including flow rates and storage values.

```

03511> =====
03512> SUM 09:POST      12.24  4.514  1.00  45.27  .000
03513>
03514> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03515>
03516> -----
03517> 001:0117-----
03518>
03519> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
03520> | IN>09:(POST ) |
03521> | OUT<01:(SWMF ) |
03522> |===== OUTFLOW STORAGE TABLE =====|
03523> | OUTFLOW STORAGE | OUTFLOW STORAGE |
03524> | (cms) (ha.m.) | (cms) (ha.m.) |
03525> | .000 .000E+00 | .079 .149E+00 |
03526> | .059 .1884E-01 | .080 .1693E+00 |
03527> | .069 .3388E-01 | .082 .1875E+00 |
03528> | .070 .4889E-01 | .083 .2073E+00 |
03529> | .072 .6439E-01 | .085 .2276E+00 |
03530> | .073 .8041E-01 | .086 .2485E+00 |
03531> | .075 .9693E-01 | .087 .2700E+00 |
03532> | .076 .1140E+00 | .299 .4003E+00 |
03533> | .078 .1316E+00 | .000 .0000E+00 |
03534>
03535> ROUTING RESULTS AREA AREA QPEAK TPEAK R.V.
03536> (ha) (cms) (hrs) (mm)
03537> INFLOW >09:(POST ) 12.24 4.514 1.000 45.274
03538> OUTFLOW<01:(SWMF ) 12.24 .181 1.983 45.276
03539> OVERFLOW<02:(000003) .00 .000 .000 .000
03540>
03541> TOTAL NUMBER OF SIMULATED OVERFLOWS = 0
03542> CUMULATIVE TIME OF OVERFLOWS (hours)= .00
03543> PERCENTAGE OF TIME OVERFLOWING (%) = .00
03544>
03545> PEAK FLOW REDUCTION [Qout/Qin] (%) = 4.017
03546> TIME SHIFT OF PEAK FLOW (min)= 59.00
03547> MAXIMUM STORAGE USED (ha.m.)=.3280E+00
03548>
03549> -----
03550> 001:0118-----
03551>
03552> | DIVERT HYD |
03553> | INID=01 (SWMF ) |
03554>
03555> Outflow / Inflow Relationships
03556> Flow 03 + Flow 04 + Flow 05 = Total
03557> (cms) (cms) (cms) (cms)
03558> .000 .000 .000 .000
03559> .000 .059 .000 .059
03560> .007 .061 .000 .069
03561> .007 .063 .000 .070
03562> .008 .064 .000 .072
03563> .008 .065 .000 .073
03564> .008 .066 .000 .075
03565> .009 .068 .000 .076
03566> .009 .069 .000 .078
03567> .009 .070 .000 .079
03568> .009 .071 .000 .080
03569> .010 .072 .000 .082
03570> .010 .073 .000 .083
03571> .010 .074 .000 .085
03572> .010 .075 .000 .086
03573> .011 .077 .000 .087
03574> .011 .078 .210 .299
03575>
03576> NHVD AREA QPEAK TpeakDate_hh:mm R.V. NFE WetHrs
03577> (ha) (cms) (hrs) (mm) (hrs)
03578> IDin = 01:SWMF 12.24 .181 No_date 1:59 45.276 1 215.
03579>
03580> IDout = 03:Infil 0.91 .011 No_date 1:59 45.276 1 13.
03581> IDout = 04:Orific 9.92 .077 No_date 1:59 45.276 1 215.
03582> IDout = 05:Emerg 1.42 .093 No_date 1:59 45.276 1 3.
03583>
03584> 001:0119-----
03585>
03586> | PRINT HYD | AREA (ha)= 1.416
03587> | ID=05 (Emerg) | QPEAK (cms)= .093 (i)
03588> | DT= 1.00 PCYC= 5 | TPEAK (hrs)= 1.983
03589> | VOLUME (mm)= 45.276
03590>
03591> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03592> TIME FLOW | TIME FLOW | TIME FLOW | TIME FLOW |
03593> hrs cms | hrs cms | hrs cms | hrs cms |
03594> .00 .000 | .83 .000 | 1.67 .087 | 2.50 .085 | 3.33 .044
03595> .08 .000 | .92 .000 | 1.75 .090 | 2.58 .083 | 3.42 .038
03596> .17 .000 | 1.00 .000 | 1.83 .092 | 2.67 .080 | 3.50 .032
03597> .25 .000 | 1.08 .000 | 1.92 .093 | 2.75 .078 | 3.58 .027
03598> .33 .000 | 1.17 .000 | 2.00 .093 | 2.83 .075 | 3.67 .021
03599> .42 .000 | 1.25 .032 | 2.08 .093 | 2.92 .072 | 3.75 .016
03600> .50 .000 | 1.33 .053 | 2.17 .092 | 3.00 .070 | 3.83 .011
03601> .58 .000 | 1.42 .066 | 2.25 .091 | 3.08 .064 | 3.92 .007
03602> .67 .000 | 1.50 .076 | 2.33 .089 | 3.17 .057 | 4.00 .003
03603> .75 .000 | 1.58 .083 | 2.42 .087 | 3.25 .051
03604>
03605> 001:0120-----
03606> *****
03607> #*
03608> #* 250-year *****
03609> #*
03610> #*
03611> #*
03612> #*
03613> | CHICAGO STORM | IDF curve parameters: A=3048.220
03614> | Ptotal= 86.60 mm | B= 10.030
03615> C= .888
03616> used in: INTENSITY = A / (t + B)^C
03617>
03618> Duration of storm = 3.00 hrs
03619> Storm time step = 5.00 min
03620> Time to peak ratio = .33
03621>
03622> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN |
03623> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr |
03624> .08 5.146 | .83 43.789 | 1.58 19.207 | 2.33 7.218
03625> .17 5.748 | .92 106.492 | 1.67 16.378 | 2.42 6.730
03626> .25 6.505 | 1.00 274.730 | 1.75 14.228 | 2.50 6.302
03627> .33 7.486 | 1.08 136.493 | 1.83 12.548 | 2.58 5.925
03628> .42 8.789 | 1.17 76.628 | 1.92 11.204 | 2.67 5.589
03629> .50 10.637 | 1.25 50.754 | 2.00 10.107 | 2.75 5.289
03630> .58 13.366 | 1.33 36.943 | 2.08 9.197 | 2.83 5.020
03631> .67 17.763 | 1.42 28.579 | 2.17 8.431 | 2.92 4.776
03632> .75 25.782 | 1.50 23.067 | 2.25 7.779 | 3.00 4.554
03633>
03634>
03635> 001:0121-----
03636> *****
03637> #*
03638> #* EXISTING CONDITIONS *****
03639> #*
03640> #*
03641> #*
03642> | CALIB NASHYD | Area (ha)= 2.37 Curve Number (CN)=72.00
03643> | 01:EX-1 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
03644> | U.H. Tp(hrs)= .330
03645>

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```

03646> Unit Hyd Qpeak (cms)= .274
03647>
03648> PEAK FLOW (cms)= .223 (i)
03649> TIME TO PEAK (hrs)= 1.433
03650> RUNOFF VOLUME (mm)= 34.829
03651> TOTAL RAINFALL (mm)= 86.599
03652> RUNOFF COEFFICIENT = .402
03653>
03654> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03655>
03656> -----
03657> 001:0122-----
03658>
03659> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
03660> | TotalHyd 01:EX-1 | Number of inlets in system [NINLET] = 1
03661> | Total minor system capacity = .001 (cms)
03662> | Total major system storage [TMJSTO] = 99999. (cu.m.)
03663>
03664> ID: NHVD AREA QPEAK TPEAK R.V. DWF
03665> (ha) (cms) (hrs) (mm) (cms)
03666> TOTAL HYD. 01:EX-1 2.37 .223 1.433 34.829 .000
03667>
03668> MAJOR SYST 02:TRAMES 1.00 .000 .000 .000 .000
03669> MINOR SYST 03:toDRAI 2.37 .001 287.433 34.829 .000
03670>
03671> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03672>
03673> Maximum MAJOR SYSTEM storage used = 816. (cu.m.)
03674>
03675> -----
03676> 001:0123-----
03677>
03678> | CALIB NASHYD | Area (ha)= .19 Curve Number (CN)=74.00
03679> | 01:EX-2 DT= 1.00 | Ia (mm)= 5.000 # of Linear Res. (N)= 3.00
03680> | U.H. Tp(hrs)= .090
03681>
03682> Unit Hyd Qpeak (cms)= .081
03683>
03684> PEAK FLOW (cms)= .041 (i)
03685> TIME TO PEAK (hrs)= 1.100
03686> RUNOFF VOLUME (mm)= 38.973
03687> TOTAL RAINFALL (mm)= 86.599
03688> RUNOFF COEFFICIENT = .450
03689>
03690> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03691>
03692> -----
03693> 001:0124-----
03694>
03695> | CALIB STANDHYD | Area (ha)= .45
03696> | 04:EX-3 DT= 1.00 | Total Imp(%) = 65.00 Dir. Conn.(%) = 1.00
03697>
03698> IMPERVIOUS PERVIOUS (i)
03699> Surface Area (ha)= .29 .16
03700> Dep. Storage (mm)= 2.00 8.00
03701> Average Slope (%) = 1.00 20.00
03702> Length (m)= 5.00 25.00
03703> Mannings n = .013 .240
03704>
03705> Max.eff.Inten.(mm/hr)= 274.73 590.27
03706> over (min) 1.00 2.00
03707> Storage Coeff. (min)= .28 (ii) 1.56 (ii)
03708> Unit Hyd. Tpeak (min)= 1.00 2.00
03709> Unit Hyd. peak (cms)= 1.65 .65
03710>
03711> PEAK FLOW (cms)= .00 .23 *TOTALS*
03712> TIME TO PEAK (hrs)= .98 1.00 (.238 (iii))
03713> RUNOFF VOLUME (mm)= 84.60 59.12
03714> TOTAL RAINFALL (mm)= 86.60 86.60 86.599
03715> RUNOFF COEFFICIENT = .98 .68 .686
03716>
03717> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
03718> CN = 72.0 Ia = Dep. Storage (Above)
03719> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
03720> THAN THE STORAGE COEFFICIENT.
03721> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03722>
03723> -----
03724> 001:0125-----
03725>
03726> | CALIB NASHYD | Area (ha)= 2.59 Curve Number (CN)=67.00
03727> | 05:EX-4 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
03728> | U.H. Tp(hrs)= .230
03729>
03730> Unit Hyd Qpeak (cms)= .430
03731>
03732> PEAK FLOW (cms)= .262 (i)
03733> TIME TO PEAK (hrs)= 1.317
03734> RUNOFF VOLUME (mm)= 30.328
03735> TOTAL RAINFALL (mm)= 86.599
03736> RUNOFF COEFFICIENT = .350
03737>
03738> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03739>
03740> -----
03741> 001:0126-----
03742>
03743> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
03744> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
03745> | Total minor system capacity = .001 (cms)
03746> | Total major system storage [TMJSTO] = 99999. (cu.m.)
03747>
03748> ID: NHVD AREA QPEAK TPEAK R.V. DWF
03749> (ha) (cms) (hrs) (mm) (cms)
03750> TOTAL HYD. 05:EX-4 2.59 .262 1.317 30.328 .000
03751>
03752> MAJOR SYST 06:BANNAN .00 .000 .000 .000 .000
03753> MINOR SYST 07:toDRAI 2.59 .001 273.550 30.329 .000
03754>
03755> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
03756>
03757> Maximum MAJOR SYSTEM storage used = 777. (cu.m.)
03758>
03759> -----
03760> 001:0127-----
03761>
03762> | CALIB NASHYD | Area (ha)= 6.64 Curve Number (CN)=72.00
03763> | 05:C101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res. (N)= 3.00
03764> | U.H. Tp(hrs)= .310
03765>
03766> Unit Hyd Qpeak (cms)= .818
03767>
03768> PEAK FLOW (cms)= .651 (i)
03769> TIME TO PEAK (hrs)= 1.417
03770> RUNOFF VOLUME (mm)= 34.829
03771> TOTAL RAINFALL (mm)= 86.599
03772> RUNOFF COEFFICIENT = .402
03773>
03774> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
03775>
03776> -----
03777> 001:0128-----
03778>
03779> | ADD HYD (PRE ) | ID: NHVD AREA QPEAK TPEAK R.V. DWF
03780> (ha) (cms) (hrs) (mm) (cms)

```

Table with 5 columns: ID, Description, Value, Unit, and another Value. Includes entries like ID1 01:EX-2, ID2 03:toDRAIN, etc.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 5 columns: ID, Description, Value, Unit, and another Value. Includes entries like PRINT HYD, AREA, QPEAK, etc.

Table with 5 columns: (i), Description, Value, Unit, and another Value. Includes entries like PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Main data table with 5 columns: Time (hrs), Flow (cms), Time (hrs), Flow (cms), Time (hrs), Flow (cms). Contains a long list of time-series data points.

Main data table with 5 columns: Time (hrs), Flow (cms), Time (hrs), Flow (cms), Time (hrs), Flow (cms). Contains a long list of time-series data points, continuing from the previous table.


```

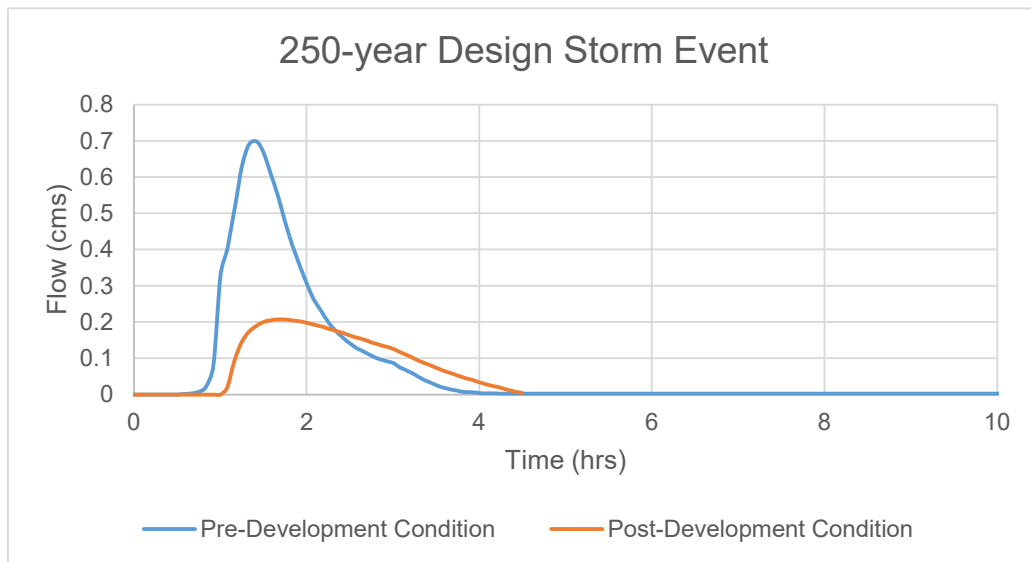
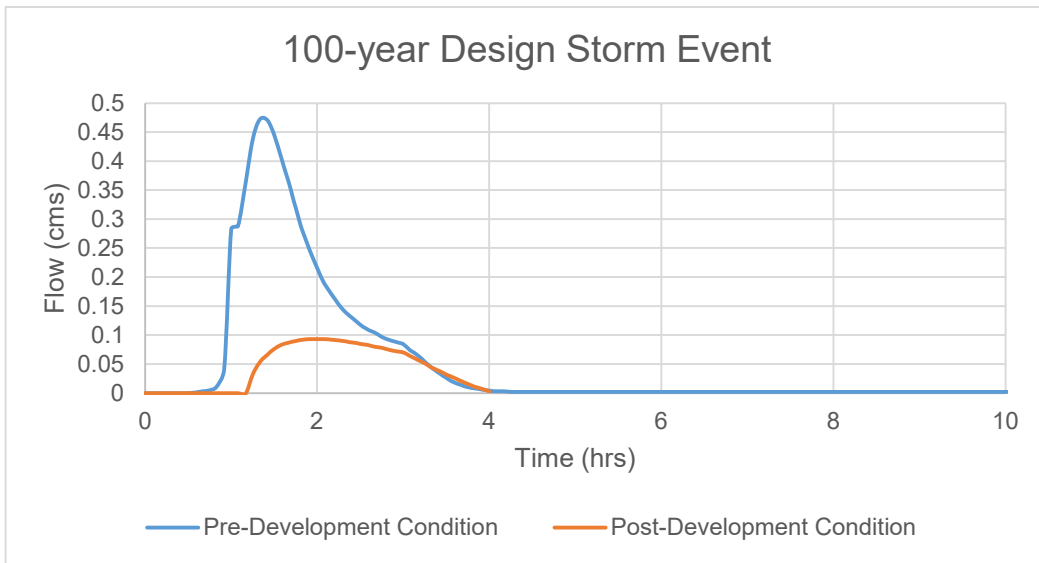
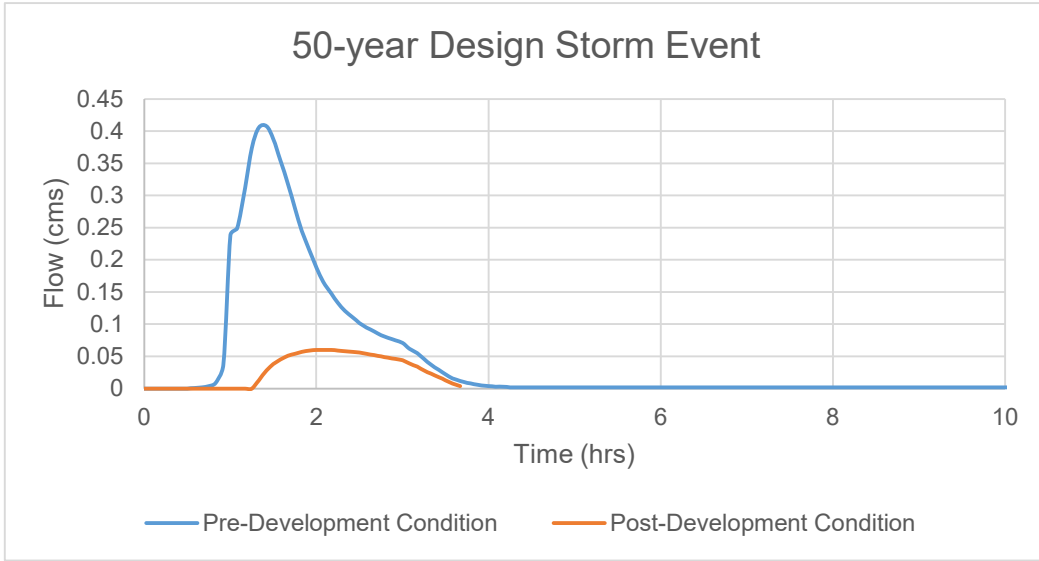
04591> TIME TO PEAK (hrs)= 1.317
04592> RUNOFF VOLUME (mm)= 30.328
04593> TOTAL RAINFALL (mm)= 86.599
04594> RUNOFF COEFFICIENT = .350
04595>
04596> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
04597>
04598>
04599> 001:0135
04600> -----
04601> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
04602> | TotalHyd 05:EX-4 | Number of inlets in system [NINLET] = 1
04603> | Total minor system capacity = .001 (cms)
04604> | Total major system storage [TMJSTO] = 99999.(cu.m.)
04605>
04606> ID: NHYD AREA QPEAK TPEAK R.V. DWF
04607> (ha) (cms) (hrs) (mm) (cms)
04608> TOTAL HYD. 05:EX-4 2.59 .262 1.317 30.328 .000
04609>
04610> MAJOR SYST 06:SANMAN .00 .000 .000 .000 .000
04611> MINOR SYST 07:toDRAI 2.59 .001 273.550 30.329 .000
04612>
04613> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
04614>
04615> Maximum MAJOR SYSTEM storage used = 777.(cu.m.)
04616>
04617> -----
04618> 001:0136
04619> -----
04620> CALIB STANDHYD | Area (ha)= 6.64
04621> | 05:C201 DT= 1.00 | Total Imp(%)= 90.00 Dir. Conn.(%)= 80.00
04622> -----
04623> IMPERVIOUS PERVIOUS (i)
04624> Surface Area (ha)= 5.98 .66
04625> Dep. Storage (mm)= 2.00 5.00
04626> Average Slope (%)= 1.00 2.00
04627> Length (m)= 50.00 25.00
04628> Mannings n = .013 .250
04629>
04630> Max.eff.Inten.(mm/hr)= 274.73 252.84
04631> over (min) 1.00 5.00
04632> Storage Coeff. (min)= 1.13 (ii) 4.80 (iii)
04633> Unit Hyd. Tpeak (min)= 1.00 5.00
04634> Unit Hyd. peak (cms)= 1.00 .23
04635>
04636> PEAK FLOW (cms)= 4.02 .33 *TOTALS*
04637> TIME TO PEAK (hrs)= 1.00 1.07 4.250 (iii)
04638> RUNOFF VOLUME (mm)= 84.60 42.79 76.237
04639> TOTAL RAINFALL (mm)= 86.60 86.60 86.599
04640> RUNOFF COEFFICIENT = .98 .49 .880
04641>
04642> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
04643> CN* = 61.0 Ia = Dep. Storage (Above)
04644> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
04645> THAN THE STORAGE COEFFICIENT.
04646> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
04647>
04648>
04649> 001:0137
04650> -----
04651> ADD HYD (POST ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
04652> (ha) (cms) (hrs) (mm) (cms)
04653> ID1 01:EX-2 .19 .041 1.10 38.97 .000
04654> +ID2 03:toDRAIN 2.37 .001 287.43 34.83 .000
04655> +ID3 04:EX-3 .45 .238 1.00 59.38 .000
04656> +ID4 05:C201 6.64 4.250 1.00 76.24 .000
04657> +IDS 07:toDRAIN 2.59 .001 273.55 30.33 .000
04658>
04659> SUM 09:POST 12.24 4.514 1.00 57.28 .000
04660>
04661> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
04662>
04663>
04664> 001:0138
04665> -----
04666> ROUTE RESERVOIR | Requested routing time step = 1.0 min.
04667> | IN>09:(POST ) |
04668> | OUT<01:(SWMF ) | ===== OUTFLOW STORAGE TABLE =====
04669> OUTFLOW STORAGE | OUTFLOW STORAGE
04670> (cms) (ha.m.) | (cms) (ha.m.)
04671> .000 .0000E+00 | .079 .1497E+00
04672> .059 .1884E-01 | .080 .1683E+00
04673> .069 .3388E-01 | .082 .1875E+00
04674> .070 .4889E-01 | .083 .2073E+00
04675> .072 .6439E-01 | .085 .2276E+00
04676> .073 .8041E-01 | .086 .2485E+00
04677> .075 .9693E-01 | .087 .2700E+00
04678> .076 .1140E+00 | .299 .4003E+00
04679> .078 .1316E+00 | .000 .0000E+00
04680>
04681> ROUTING RESULTS AREA QPEAK TPEAK R.V.
04682> (ha) (cms) (hrs) (mm)
04683> INFLOW<09:(POST ) 12.24 4.514 1.000 57.283
04684> OUTFLOW<01:(SWMF ) 12.24 .296 1.683 57.285
04685> OVERFLOW<02:(000003) .00 .000 .000 .000
04686>
04687> TOTAL NUMBER OF SIMULATED OVERFLOWS = 0
04688> CUMULATIVE TIME OF OVERFLOWS (hours)= .00
04689> PERCENTAGE OF TIME OVERFLOWING (%)= .00
04690>
04691>
04692> PEAK FLOW REDUCTION [Qout/Qin] (%)= 6.568
04693> TIME SHIFT OF PEAK FLOW (min)= 41.00
04694> MAXIMUM STORAGE USED (ha.m.)=.3988E+00
04695>
04696> -----
04697> 001:0139
04698> -----
04699> DIVERT HYD |
04700> | INID=01 (SWMF ) |
04701> -----
04702> Outflow / Inflow Relationships
04703> Flow 03 + Flow 04 + Flow 05 = Total
04704> (cms) (cms) (cms) (cms)
04705> .000 .000 .000 .000
04706> .000 .059 .000 .059
04707> .007 .061 .000 .069
04708> .007 .063 .000 .070
04709> .008 .064 .000 .072
04710> .008 .065 .000 .073
04711> .008 .066 .000 .075
04712> .009 .068 .000 .076
04713> .009 .069 .000 .078
04714> .009 .070 .000 .079
04715> .009 .071 .000 .080
04716> .010 .072 .000 .082
04717> .010 .073 .000 .083
04718> .010 .074 .000 .085
04719> .010 .075 .000 .086
04720> .011 .077 .000 .087
04721> .011 .078 .210 .299
04722>
04723> NHYD AREA QPEAK TpeakDate_hh:mm R.V. NFE WetHrs
04724> (ha) (cms) (hrs) (mm) (hrs)
04725> Idin = 01:SWMF 12.24 .296 No_date 1:41 57.285 1 295.

```

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04726> =====
04727> IDout= 03:Infill .75 .011 No_date 1:41 57.285 1 13.
04728> IDout= 04:Orific 8.90 .078 No_date 1:41 57.285 1 295.
04729> IDout= 05:Emerge 2.58 .208 No_date 1:41 57.285 1 4.
04730>
04731> 001:0140
04732> -----
04733> PRINT HYD | AREA (ha)= 2.583
04734> | ID=05 (Emerge) | QPEAK (cms)= 1.208 (i)
04735> | DT= 1.00 PCYC= 5 | TPEAK (hrs)= 1.683
04736> | VOLUME (mm)= 57.285
04737>
04738> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
04739> TIME FLOW | TIME FLOW | TIME FLOW | TIME FLOW | TIME FLOW
04740> hrs cms | hrs cms | hrs cms | hrs cms | hrs cms
04741> .00 .000 | 1.00 .000 | 2.00 .198 | 3.00 .126 | 4.00 .034
04742> .08 .000 | 1.08 .020 | 2.08 .193 | 3.08 .117 | 4.08 .028
04743> .17 .000 | 1.17 .099 | 2.17 .188 | 3.17 .108 | 4.17 .023
04744> .25 .000 | 1.25 .145 | 2.25 .182 | 3.25 .099 | 4.25 .018
04745> .33 .000 | 1.33 .173 | 2.33 .176 | 3.33 .090 | 4.33 .013
04746> .42 .000 | 1.42 .190 | 2.42 .170 | 3.42 .082 | 4.42 .008
04747> .50 .000 | 1.50 .200 | 2.50 .163 | 3.50 .074 | 4.50 .004
04748> .58 .000 | 1.58 .205 | 2.58 .157 | 3.58 .066
04749> .67 .000 | 1.67 .207 | 2.67 .151 | 3.67 .059
04750> .75 .000 | 1.75 .207 | 2.75 .144 | 3.75 .052
04751> .83 .000 | 1.83 .205 | 2.83 .138 | 3.83 .046
04752> .92 .000 | 1.92 .202 | 2.92 .132 | 3.92 .040
04753>
04754> 001:0141
04755> -----
04756> FINISH
04757> -----
04758> WARNINGS / ERRORS / NOTES
04759> -----
04760> Simulation ended on 2023-11-21 at 14:53:40
04761> -----
04762>
04763>

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APPENDIX C
SUPPORTING CALCULATIONS

Upstream of Maintenance Hole R20

Pipe Storage			
Description	Pipe Size (mm)	Length (m)	Volume (m ³)
R20 - R9	750	42.9	18.95
R9 - CB	300	21.5	1.52
R9 - R8	600	28.3	8.00
R8 - R7	600	32.8	9.27
R7 - R6	600	37.0	10.46
R6 - RYCB	300	52.3	3.70
R6 - R5	600	8.3	2.35
R5 - R4	525	25.5	5.52
R4 - R3	525	35.0	7.58
R3 - R2	450	41.6	6.62
R2 - R1	300	25.9	1.83
R20 - R19	750	28.0	12.37
R19 - R18	750	32.5	14.36
R18 - R17	750	71.6	31.63
R17 - R16	750	35.9	15.86
R16 - R15	750	30.6	13.52
R15 - CB	300	17.8	1.26
R15 - R14	600	45.0	12.72
R14 - R13	525	31.6	6.84
R13 - R12	525	11.5	2.49
R12 - R11	450	52.9	8.41
R12 - RYCB	300	77.2	5.46
R11 - R10	300	8.3	0.59
TOTAL			146

Structure Storage					
Structures	Diameter (mm)	Invert (m)	Lid (m)	Height (m)	Vol @ Max Ponding
R1	1200	234.42	236.69	2.23	2.52
R2	1200	234.25	236.80	2.40	2.71
R3	1200	234.07	236.59	2.52	2.85
R4	1200	233.94	236.41	2.47	2.79
R5	1200	233.83	236.55	2.72	3.08
R6	1200	233.78	236.61	2.83	3.20
R7	1200	233.64	236.53	2.89	3.27
R8	1200	233.45	236.37	2.92	3.30
R9	1500	233.28	236.50	3.22	5.69
R10	1200	234.06	236.81	2.59	2.93
R11	1200	233.99	236.74	2.66	3.01
R12	1200	233.77	236.47	2.70	3.05
R13	1200	233.70	236.41	2.71	3.06
R14	1200	233.56	236.57	3.01	3.40
R15	1800	233.37	236.57	3.20	8.14
R16	1500	233.28	236.37	3.09	5.46
R17	1500	233.18	236.54	3.36	5.94
R18	1500	233.05	236.48	3.43	6.06
R19	1500	232.93	236.32	3.39	5.99
R20	1800	232.83	236.52	3.69	9.39
TOTAL					86

Max Ponding (m)	236.65	Total Storage Volume Available (m³)	232
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SWMF Stage-Storage Information

Stage (m)	Depth (m)	Main Pond			Underground Storage		Surface Ponding	Total Pond Volume (m ³)	Comments	Stage (m)
		Area (m ²)	Volume (m ³)	Cumulative Volume (m ³)	Incremental Volume (m ³)	Cumulative Volume (m ³)	Volume (m ³)			
235.00	0.00	0	0	0	188	188	0	188	Bottom of Dry Pond	235.00
235.20	0.20	1447	145	145	6	194	0	339		235.20
235.30	0.30	1497	147	292	3	197	0	489		235.30
235.40	0.40	1547	152	444	3	200	0	644		235.40
235.50	0.50	1598	157	601	3	203	0	804		235.50
235.60	0.60	1650	162	764	3	206	0	969		235.60
235.70	0.70	1702	168	931	3	208	0	1140		235.70
235.80	0.80	1756	173	1104	3	211	0	1316		235.80
235.90	0.90	1810	178	1283	3	214	0	1497		235.90
236.00	1.00	1864	184	1466	3	217	0	1683		236.00
236.10	1.10	1920	189	1655	3	220	0	1875		236.10
236.20	1.20	1976	195	1850	3	223	0	2073		236.20
236.30	1.30	2033	200	2051	3	226	0	2276		236.30
236.40	1.40	2090	206	2257	3	228	0	2485		236.40
236.50	1.50	2166	213	2470	2	230	0	2700	Top of Pond	236.50
236.65	1.65	0	0	2470	1	232	1302	4003	Emergency Overland Spill Point	236.65



SWMF
Stage-Storage-Discharge
Information

10919 Longwoods Road Light Industrial
 Project Number: LD-00318
 2023-11-20

Orifice Calculation			
$Q_o = C_d A \sqrt{2gH_o}$			
	Orifice 1	Orifice 2	Orifice 3
C _d	0.63	0.63	-
Invert (m)	232.83	236.55	-
Diameter/Height (m)	0.135	0.55	-
Area(m ²)	0.014	0.238	-

Infiltration Rate	
Infiltration Rate (mm/hr)	18
Bottom Contact Area (m ²)	1447
Top Contact Area (m ²)	2166

Stage	Infiltration	Orifice 1		Orifice 2		Total Flow (m ³ /s)	Active Volume	
	Flow (m ³ /s)	H _o (m)	Flow (m ³ /s)	H _o (m)	Flow (m ³ /s)		(m ³)	(ha-m)
235.00	0.000	2.17	0.059	0.00	0.000	0.059	188	0.01884
235.20	0.007	2.37	0.061	0.00	0.000	#REF!	339	0.03388
235.30	0.007	2.47	0.063	0.00	0.000	0.070	489	0.04889
235.40	0.008	2.57	0.064	0.00	0.000	0.072	644	0.06439
235.50	0.008	2.67	0.065	0.00	0.000	0.073	804	0.08041
235.60	0.008	2.77	0.066	0.00	0.000	0.075	969	0.09693
235.70	0.009	2.87	0.068	0.00	0.000	0.076	1140	0.11398
235.80	0.009	2.97	0.069	0.00	0.000	0.078	1316	0.13156
235.90	0.009	3.07	0.070	0.00	0.000	0.079	1497	0.14967
236.00	0.009	3.17	0.071	0.00	0.000	0.080	1683	0.16833
236.10	0.010	3.27	0.072	0.00	0.000	0.082	1875	0.18753
236.20	0.010	3.37	0.073	0.00	0.000	0.083	2073	0.20730
236.30	0.010	3.47	0.074	0.00	0.000	0.085	2276	0.22762
236.40	0.010	3.57	0.075	0.00	0.000	0.086	2485	0.24850
236.50	0.011	3.67	0.077	0.00	0.000	0.087	2700	0.26999
236.65	0.011	3.82	0.078	0.10	0.210	0.299	4003	0.40033

APPENDIX D
OGS SIZING SHEET

ADS OGS Sizing Summary

Project Name:	10919 Longwoods Rd	
Consulting Engineer:	LDS Consultants Inc.	
Location:	Delaware, ON	
Sizing Completed By:	C. Neath	Email: cody.neath@ads-pipe.com

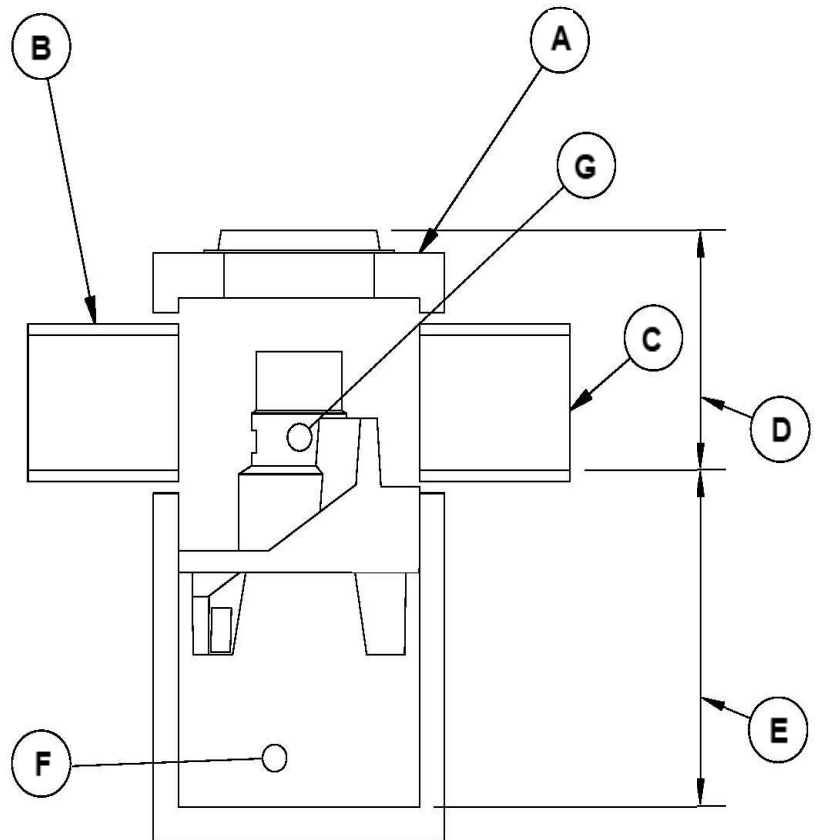
Treatment Requirements		
Treatment Goal:	Enhanced (MOE)	
Selected Parameters:	80% TSS	90% Volume
Selected Unit:	FD-8HC	

Site Details	
Site Area:	12.25 ha
% Impervious:	44%
Rational C:	0.59
Rainfall Station:	London Intl Airport, ON
Particle Size Distribution:	Fine
Peak Flowrate:	---

Summary of Results		
Model	TSS Removal	Volume Treated
FD-4HC	70.0%	83.6%
FD-5HC	74.0%	>90%
FD-6HC	76.0%	>90%
FD-8HC	80.0%	>90%
FD-10HC	83.0%	>90%

FD-8HC Specification	
Unit Diameter (A):	2,400 mm
Inlet Pipe Diameter (B):	600 mm
Outlet Pipe Diameter (C):	600 mm
Height, T/G to Outlet Invert (D):	2500 mm
Height, Outlet Invert to Sump (E):	2260 mm
Sediment Storage Capacity (F):	3.47 m ³
Oil Storage Capacity (G):	4,239 L
Recommended Sediment Depth for Maintenance:	465 mm
Max. Pipe Diameter:	1,200 mm
Peak Flow Capacity:	1,415 L/s

Site Elevations:	
Rim Elevation:	100.00
Inlet Pipe Elevation:	97.50
Outlet Pipe Elevation:	97.50



Notes:

Removal efficiencies are based on NJDEP Test Protocols and independently verified.

All units supplied by ADS have numerous local, provincial, and international certifications (copies of which can be provided upon request). The design engineer is responsible for ensuring compliance with applicable regulations.



Project Name: 10929 Longwoods Rd
 Consulting Engineer: LDS Consultants Inc.
 Location: Delaware, ON

Net Annual Removal Efficiency Summary: FD-8HC

Rainfall Intensity ⁽¹⁾	Fraction of Rainfall ⁽¹⁾	FD-8HC Removal Efficiency ⁽²⁾	Weighted Net-Annual Removal Efficiency
mm/hr	%	%	%
0.50	0.2%	95.5%	0.2%
1.00	13.7%	89.5%	12.3%
1.50	17.3%	86.2%	14.9%
2.00	13.5%	83.9%	11.4%
2.50	2.7%	82.2%	2.2%
3.00	2.3%	80.8%	1.8%
3.50	8.5%	79.7%	6.8%
4.00	4.7%	78.7%	3.7%
4.50	1.5%	77.8%	1.1%
5.00	5.2%	77.1%	4.0%
6.00	4.1%	75.8%	3.1%
7.00	4.4%	74.7%	3.3%
8.00	3.3%	73.8%	2.4%
9.00	2.4%	73.0%	1.8%
10.00	2.3%	72.3%	1.7%
20.00	9.2%	67.7%	6.2%
30.00	2.5%	65.2%	1.6%
40.00	1.1%	63.5%	0.7%
50.00	0.4%	62.2%	0.3%
100.00	0.6%	58.3%	0.4%
150.00	0.1%	56.2%	0.1%
200.00	0.0%	54.7%	0.0%
Total Net Annual Removal Efficiency:			79.9%
Total Runoff Volume Treated:			>90%

Notes:

- (1) Rainfall Data: 1960:2002, HLY03, London AP, ONT, 6144475.
- (2) Based on third party verified data and approximating the removal of a PSD similar to the STC Fine distribution
- (3) Rainfall adjusted to 5 min peak intensity based on hourly average.

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