



MTE Consultants

123 St. George St., London, Ontario N6A 3A1

July 13, 2020

MTE File No.: C45013-200

County of Middlesex
399 Ridout Street North
London, ON N6A 2P1

Attention: Durk Vanderwerff, Director of Planning

**RE: Preliminary Stormwater Management Report
For Vacant Land Condominium Development (Light Industrial Sites)
at 10919 Longwood Road, Delaware**

Introduction:

It is proposed to develop the property at 10919 Longwood Road, Delaware in the Municipality of Middlesex Center. The development property is 6.64 Ha in size and is located south of Longwoods Road on the east side of Delaware. The property is zoned for light industrial use. It is proposed to develop the property into 15 individual lots for light industrial use. The lots will range from 0.30 to 0.46 Ha in size. The lots will be created through a vacant land condominium which will contain a private internal roadway to provide access to the lots and a private stormwater management (SWM) basin which will provide a stormwater outlet for the lots. The private roadway and SWM basin will be common elements within the condo development.

Please refer to the attached Draft Plan of Vacant Land Condominium (DWG-1) which shows the proposed development. This letter report outlines the proposed storm drainage and stormwater management for the development in support of draft plan approval.

Pre-development Conditions:

The pre-development site is farmland. The pre-development drainage patterns are outlined below:

- The southerly 1.8 Ha of development drains to the south onto the adjacent farm.
- The remainder 4.85 Ha of the development drains to multiple low lying areas throughout the property where the runoff infiltrates into the ground.

MTE Consulting completed a detailed geotechnical investigation for the property. The existing soils are a mixture of sand and silt. (Reference: Geotechnical Investigation for Proposed Industrial Subdivision at 10919 Longwoods Road, MTE Consultants Inc., March 27, 2019).

Proposed Post-Development Site:

Please refer to the attached Draft Plan of Vacant Land Condominium (DWG-1) and the Preliminary Grading Plan (DWG-2). It is proposed to drain the development as follows:

- Each lot will be individually serviced with internal storm sewers and/or ditches which will discharge stormwater runoff to the ditching system located within the private roadway corridor.
- The private roadway corridor will contain roadside ditches on both sides of the asphalt road. The ditches will be approximately 0.80m deep with 4:1 side slopes and a 1m wide flat bottom. The ditches will be grassed with a minimum slope of 0.50% and will drain into the north portion of the proposed SWM basin. Culverts will be installed at the intersection and at future driveways.
- The rear of lots 8-15 will drain into a rear yard swale system, located at the south edge of the development. These swales will be grassed with a 0.50% minimum slope and will drain into the south portion of the proposed SWM basin.
- The roadway ditches and rear yard swales will all outlet a private SWM basin which will be located between lots 13 and 14. The SWM basin will be 0.51 Ha in size and will infiltrate stormwater runoff into the ground in order to match and mimic the pre-development condition.

Infiltration Basin Design:

MTE Consulting completed on-site infiltration testing and determined that the factored infiltration rate of the sandy soils at the infiltration basin location is 18mm/hr. The infiltration rate contains a factor of safety of 3. (Reference: Letter Results of In-situ Infiltration Testing, Proposed Industrial Subdivision, 10919 Longwoods Road, Middlesex Centre, MTE Consultants Inc., July 3, 2020). The following details outline the proposed design of the infiltration basin:

- Drainage Area = 6.65 Ha (including the area of the basin itself)
- Bottom Area of Basin = 0.28 Ha
- Top Area of Basin (to top of slope) = 0.43 Ha
- Block Size = 0.51 Ha
- Side slopes = 4:1 max
- Bottom and sides of infiltration basin will be topsoil and grassed
- Factored infiltration rate = 18mm/hr (contains a factor of safety of 3)
- Total infiltration rate for facility $0.28 \text{ Ha} \times 10,000\text{m}^2/\text{Ha} \times 18\text{mm/hr} / 1000 = 50.4\text{m}^3/\text{hr} = 0.014\text{m}^3/\text{sec}$
- Estimated maximum ground water table elevation = 233.60
- Proposed bottom of basin = 234.80
- Estimated clearance (bottom of basin to estimated ground water) = 1.20m

Hydraulic Modelling for Infiltration Basin:

The post-development tributary area to the infiltration basin is outlined below:

- Private Light Industrial Lots = 4.96 Ha (maximum impervious area = 60%)
- Private Roadway Corridor with ditches = 1.17 Ha (impervious area = 45%)
- Private SWM Facility = 0.51 Ha (equivalent impervious area = 50%)
- Total Area = 6.65 Ha, Composite Impervious = 58%
- To be conservative, we have used an overall Composite Impervious Ratio of 60%

As noted on the Preliminary Grading Plan (DWG-2), there appear to be areas from Longwood Road which drain onto the development lands. As shown on the plan, we anticipate and recommend the drainage from the road allowance area be conveyed to the east/west such that the road allowance runoff does not drain onto the private development.

We have completed hydraulic models for the inflow/outflow/ponding of the infiltration basin using SWMHYMO-99. The City of London IDF curves were used to model the following storm events:

- 25mm, 4 hour Chicago Storm
- 1:2 year, 4 hour Chicago Storm
- 1:5 year, 4 hour Chicago Storm
- 1:10 year, 4 hour Chicago Storm
- 1:50 year, 4 hour Chicago Storm
- 1:100 year, 4 hour Chicago Storm

To be conservative, we have used a 4 hour storm duration instead of a 3 hour duration.

As noted previously, the pond infiltration rate (outflow rate) was set at 0.014m³/s which contains a factor of safety of 3.

The attached Table 1 outlines the infiltration basin storage volumes and associated draw down times during each storm event. Detailed SWMHYMO-99 output is also attached.

Infiltration Basin Ponding Depths and Surface Outflow:

Please refer to Table 1 for the estimated ponding depths for each storm event. Please also refer to the attached Pond Cross Section (DWG-3).

The bottom of the basin has been set at 234.80.

1:2 year storm estimated ponding elevation is 235.40 (depth = 0.60m). Small amounts of ponding of 0.30m will occur in isolated areas in the adjacent roadside ditches to an elevation 235.40. To be conservative, storage volumes and infiltration for the roadside ditching has not been factored into this analysis.

The 1:100 year storm estimated ponding elevation is 235.95 (depth = 1.15m). The maximum grade around of the SWM basin is 236.40. As such there is 0.45m freeboard. Ponding of 0.85m will also occur in the roadside ditch to elevation 235.95. However, the majority of the ditch (average elevation of 359.90) will only have 0.05m ponding.

An emergency overflow to the south is provided at elevation 236.20, which is 0.25m higher than the facility storage level for the 1:100 year storm event.

The infiltration basin has been designed to infiltration upto the 1:100 year storm event. As such there should be no surface run-off onto adjacent properties, under normal operating conditions.

Quality Control:

As previously outlined, the development will consist of 15 light industrial lots. The development of each lot will be subject to future Site Plan approvals which will require detailed engineering, grading and stormwater management plans be provided for each lot. Each lot will need to provide an OGS unit such that quality control for the development is provided. The OGS units shall be designed for Level 1 protection. Depending on the end use of each lot, lots with less than 250m² of total asphalt area may be able to use swales and landscape treatment instead of an oil grit separator. This will be determined and assessed on an individual lot basis during the site plan process for each lot.

Construction Staging for Infiltration Basin:

The infiltration basin should be completed at the initial stages of the development. Active sediment and erosion controls shall be implemented to decrease sediment load into the basin during construction. After the overall site is 85% built out (i.e. 85% of the individual lots are completed including asphalt, vegetation and sodding), the pond will be sub-excavated to remove any accumulated silt. After 85% built-out the following will occur:

- MTE Consulting will inspect the pond bottom and take soil samples.
- The basin bottom including the bottom 2m of each side slope will be sub-excavated to 0.30m minimum depth. The excavated material shall be removed from the site and replaced with 0.30m depth of imported clean sand (no silt/clay). The purpose of this operation is to remove any contaminated sand, silt and sediment that has accumulated in the pond during development construction.
- A 150mm depth of sandy topsoil mix shall be installed on top of the new sand layer.
- The native sand and the imported 0.30m of sand should not be compacted and the top layers of both the native sand and the imported sand shall be scarified prior to the installation of the topsoil mix.

Topsoil and Seeding for Infiltration Basin:

Topsoil and seed shall be installed on the basin bottom and the 4:1 side slopes. It is important that the topsoil is very pervious and a 30/70 sand to topsoil mix shall be used for the topsoil layer within the basin. The topsoil layer will have a maximum thickness of 150mm and shall be seeded (not sodded). The seeding mix shall be Native Prairie Meadow Mix.

Initial Monitoring of the Infiltration Basin:

After the overall development is 85% built out and the basin seeding has been established, MTE Consulting will monitor the basin for a 1 year period. The purpose of the monitoring is to ensure the basin is functioning properly and within the design parameters. If the basin is not functioning properly MTE will provide recommendations for any remediation work.

On-going Maintenance of Infiltration Basin:

The infiltration basin will be privately owned and operated by the condo board. The condo board shall complete yearly monitoring and maintenance of the infiltration basin.

The condo board shall undertake the following maintenance and shall allow for the associated costs in the condo board reserve fund:

- The grass areas within the pond shall be cut on a regular basis and pond area shall be kept free of garbage and debris.
- Every five (5) years, the basin bottom and bottom 2m of the side slopes shall be fully excavated by 0.30m. The 0.15m thickness of topsoil and 0.15m thickness of sand below the topsoil shall be removed and replaced with new clean sand and topsoil (with no clay/silt content). The replacement of these materials every 5 years will be required in order to keep the infiltration basin functioning.

Conclusions

This letter report outlines the proposed storm drainage and stormwater management for the development in support of the draft plan approval. We anticipated an updated report will be prepared as a condition of draft plan approval.

In summary, the development consists 15 individual lots for light industrial use which will be development through a vacant land condominium. The stormwater drainage for the development will outlet to a stormwater infiltration basin. The infiltration basin is 0.51 Ha in size and has been designed to accommodate the 1:100 post-development flows from the development. The infiltration basin, private roadway and conveyance ditches will be privately owned and maintained by the condominium board.

Level 1 stormwater quality controls will be provided on the individual lots prior to discharge to the internal roadway ditch system. The development of the individual lots will be subject to future site plan approvals which will require detailed engineering, grading and stormwater management plans be provided for each lot.

Please contact us if you have any comments or questions.

Yours Truly,

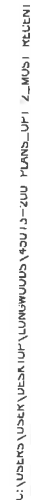
MTE Consultants Inc.



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kmcintosh@mte85.com

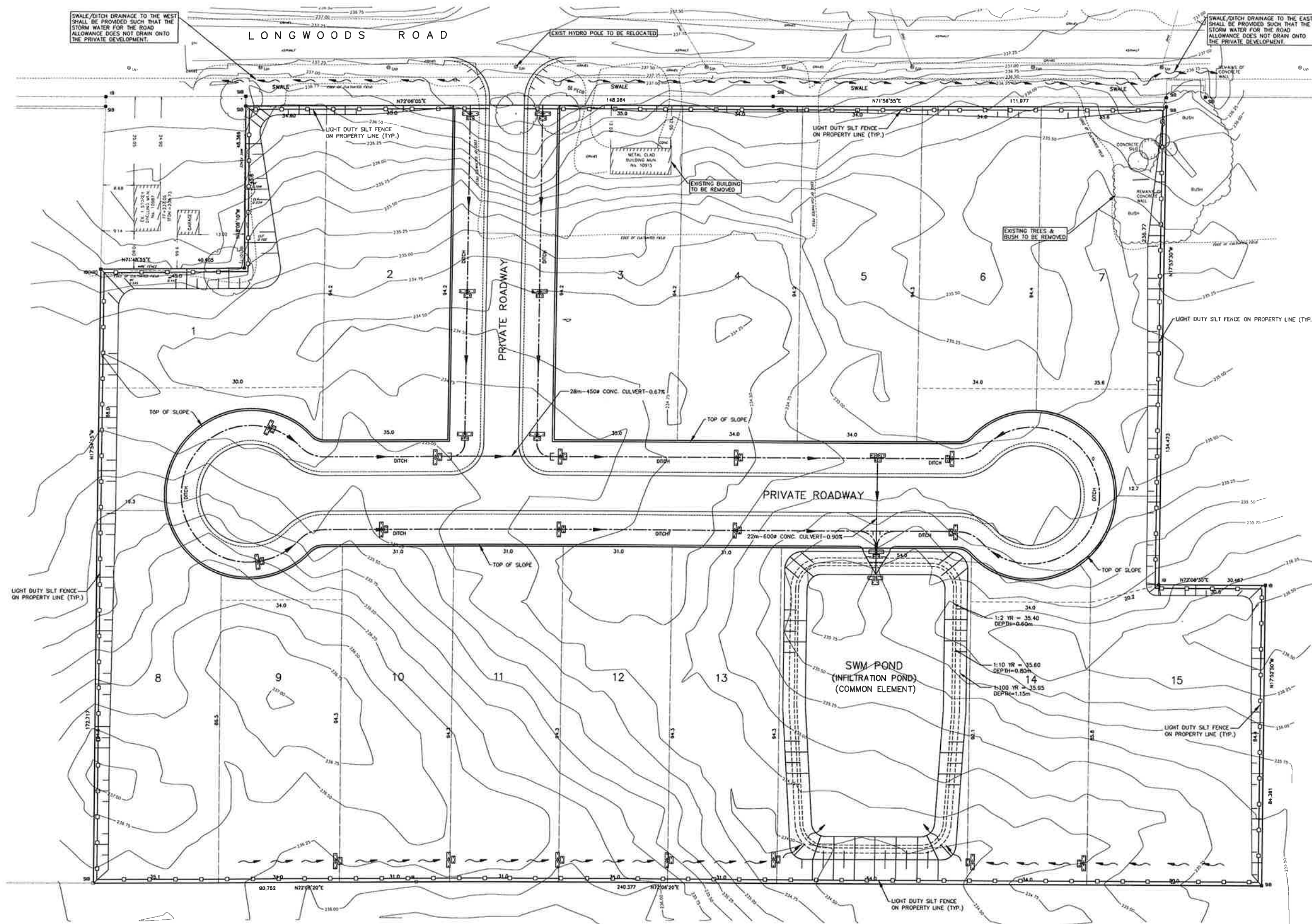
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Table 1: Infiltration Basin Calculations						
Design Storm	Runoff volume to infiltration basin (m3)	Infiltration Basin Infiltration Rate (m3/hour)	Infiltration Amount During Storm (m3)	Infiltration Basin Total Storage Required (m3)	Drain down time after storm event (hours)	Infiltration Basin Estimated Ponding Depth (m)
25mm, 4 hours	918	50	202	716	14	0.35
1:2 year, 4 hours	1915	50	202	1713	34	0.6
1:5 year, 4 hours	2006	50	202	1804	36	0.7
1:10 year, 4 hours	2447	50	202	2245	45	0.8
1:50 year, 4 hours	3425	50	202	3223	64	0.95
1:100 year, 4 hours	3853	50	202	3651	72	1.15

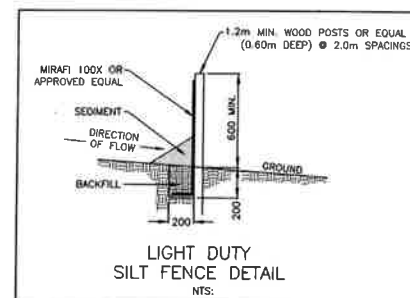
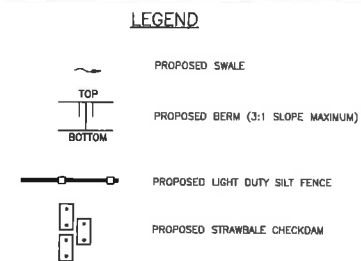
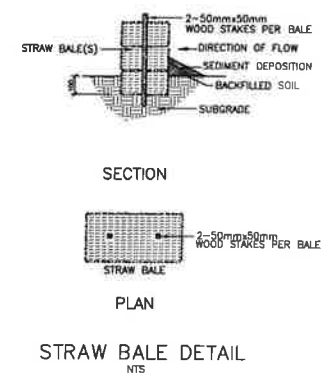


EXISTING SERVICES	DRAWING #, SOURCE	CONSTRUCTION DATE	CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT	CONSULTANT OR DESIGN	ENGINEER'S STAMP	SCALE	TITLE	PROJECT No.
					DESIGN KAM					 MTE Engineers, Scientists, Surveyors		SCALE - AS SHOWN	LOT 6, CONCESSION 1 MUNICIPALITY OF MIDDLESEX CENTRE DETAILS	45013-200
					DRAWN BY WA				SHEET No.					
					CHECKED KAM				3					
					APPROVED KAM				PLAN FILE No.					
					DATE JULY 28									

C:\USERS\USER\DESKTOP\LONGWOODS\2013-2014 PLANS\DWG 2.MSI REVEN



- SEDIMENT CONTROL MEASURES**
1. INSTALL SILT FENCE AROUND PERIMETER OF ENTIRE SITE. INSTALL STRAW BALE CHECK DAMS AT ALL CBs IN GRASS AREAS.
 2. PROTECT ALL EXPOSED SURFACES AND CONTROL ALL RUNOFF DURING CONSTRUCTION.
 3. ALL EROSION CONTROL MEASURES ARE TO BE IN PLACE BEFORE STARTING CONSTRUCTION AND REMAIN IN PLACE UNTIL RESTORATION IS COMPLETE.
 4. MAINTAIN EROSION CONTROL MEASURES DURING ALL PHASES OF CONSTRUCTION.
 5. ALL COLLECTED SEDIMENT IS TO BE DISPOSED OF AT AN APPROVED LOCATION.
 6. PROTECT ALL SITE CATCHBASINS, MANHOLES AND PIPE ENDS FROM SEDIMENT INTRUSION WITH GEOTEXTILE (TERRAFIX 270R).
 7. PREVENT WIND BLOWN DUST.
 8. STRAW BALES ARE TO BE USED AT ALL CB/CBMH AND IN LOCALIZED AREAS AS DIRECTED BY THE ENGINEER DURING CONSTRUCTION.
 9. WHEN ALL RESTORATION IS COMPLETED REMOVE SEDIMENT CONTROL MEASURES AND RESTORE IMPACTED AREAS.
 10. ALL SILT FENCING AND DETAILS ARE AT THE MINIMUM TO BE CONSTRUCTED IN ACCORDANCE WITH THE MINISTRY OF NATURAL RESOURCES GUIDELINES ON EROSION AND SEDIMENT CONTROL FOR URBAN CONSTRUCTION SITES.
 11. ALL OF THE ABOVE NOTES AND ANY SEDIMENT AND EROSION CONTROL MEASURES ARE AT THE MINIMUM TO BE IN ACCORDANCE WITH THE MINISTRY OF NATURAL RESOURCES GUIDELINES ON EROSION AND SEDIMENT CONTROL FOR URBAN CONSTRUCTION SITES.



EXISTING SERVICES	DRAWING #, SOURCE	CONSTRUCTION DATE	CONSTRUCTED SERVICES	COMPLETION DATE	DETAILS	No.	REVISIONS	DATE	CONSULTANT
					DESIGN BY: KAM				
					CHECKED BY: KAM				
					APPROVED BY: KAM				
					DATE: JUL/20				



ENGINEER'S STAMP

SCALE
SCALE = 1 : 500
5 0 10m

LOT 6, CONCESSION 1
MUNICIPALITY OF MIDDLESEX CENTRE
**PRELIMINARY EROSION
CONTROL PLAN**

PROJECT No. 45013-200
SHEET No. 4
PLAN FILE No.

START | Project dir.: Q:\45013\200\SWMHYMO\60%TMP-1\1
Rainfall dir.: Q:\45013\200\SWMHYMO\60%TMP-1\1
TZERO = .00 hrs on 0
METOUT = 2 (output = METRIC)
NRUN = 002
NSTORM = 1
1=2YR HYT

002:0002
Project Name: [10919 LONGWOODS ROAD INDUSTRIAL SUBDIVISION] Project Numbe
Date: MARCH 2020
Modeler: [MFM]
Company: MTE CONSULTANTS INC.
License #

002:0002
READ STORM | Filename: 2YR Chicago 4-hr duration
Ptotal= 44.77 mm | Comments: 2YR Chicago 4-hr duration

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs
.08	1.904	1.17	6.297	2.25	8.958	3.33	7.670
.17	2.008	1.25	7.670	2.33	7.670	3.42	2.669
.25	2.126	1.33	9.770	2.42	6.698	3.50	2.524
.33	2.259	1.42	13.315	2.50	5.940	3.58	2.410
.42	2.410	1.50	20.302	2.58	5.334	3.67	2.307
.50	2.585	1.58	38.522	2.67	4.840	3.75	2.213
.58	2.787	1.67	131.990	2.75	4.430	3.83	2.126
.67	3.026	1.75	77.410	2.83	4.083	3.92	2.046
.75	3.310	1.83	38.522	2.92	3.788	4.00	1.972
.83	3.656	1.92	24.316	3.00	3.533	4.08	1.904
.92	4.083	2.00	17.342	3.08	3.310		
1.00	4.626	2.08	13.315	3.17	3.115		
1.08	5.334	2.17	10.734	3.25	2.942		

PROPOSED SITE

*AREA 201 -> PROPOSED LONGWOODS ROAD INDUSTRIAL SUBDIVISION

CALIB STANDHYD	Area	(ha)	Total Imp(%)	66.5	Dir.	Conn. (%)	50.00
01:201	DT= 5.00						
Surface Area	(ha)	131.99	45.94				
Dep. Storage	(mm)	3.99	2.66				
Average Slope	(%)	2.00	5.00				
Length	(m)	80.00	2.50				
Mannings n		.015	.035				
Max. eff. Inten. (mm/hr)		131.99	45.94				
Storage Coeff. (min)		5.00	5.00				
Unit Hyd. Tpeak (min)		2.18 (ii)	6.38 (ii)				
Unit Hyd. peak (cms)		.31	.18				
PEAK FLOW (cms)		1.13	.25				
TIME TO PEAK (hrs)		1.67	1.75				
RUNOFF VOLUME (mm)		42.77	14.82				
TOTAL RAINFALL (mm)		44.77	44.766				
RUNOFF COEFFICIENT		.96	.33				

*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

002:0004

COMPUTE VOLUME
ID:01 (201)
DISCHARGE
(cms)
START CONTROLLING AT
INFLOW HYD. PEAKS AT
STOP CONTROLLING AT
REQUIRED STORAGE VOLUME (ha.m.) = .1808
TOTAL HYDROGRAPH VOLUME (ha.m.) = .1915
% OF HYDROGRAPH TO STORE = 94.4373

NOTE: Storage was computed to reduce the Inflow
peak to .017 (cms).

002:0005

002:0002
** END OF RUN : 2

START | Project dir.: Q:\45013\200\SWMHYMO\60%TMP-1\1
Rainfall dir.: Q:\45013\200\SWMHYMO\60%TMP-1\1
TZERO = .00 hrs on 0
METOUT = 2 (output = METRIC)
NRUN = 003
NSTORM = 1
1=5YR HYT

003:0002

Project Name: [10919 LONGWOODS ROAD INDUSTRIAL SUBDIVISION] Project Numbe
Date: MARCH 2020
Modeler: [MFM]
Company: MTE CONSULTANTS INC.
License #

003:0002
READ STORM | Filename: 5YR Chicago 4-hr duration
Ptotal= 46.48 mm | Comments: 5YR Chicago 4-hr duration

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs
.08	2.193	1.17	8.715	2.25	9.452	3.33	3.145
.17	2.303	1.25	8.715	2.33	9.452	3.42	2.997
.25	2.435	1.33	9.761	2.42	6.162	3.50	2.863
.33	2.578	1.42	13.261	2.50	6.413	3.58	2.742
.42	2.742	1.50	20.302	2.58	5.795	3.67	2.631
.50	2.926	1.58	38.522	2.67	5.288	3.75	2.559
.58	3.148	1.67	131.791	2.75	4.863	3.83	2.488
.67	3.398	1.75	77.604	2.83	4.506	3.92	2.435
.75	3.698	1.83	38.554	2.92	4.268	4.00	2.388
.83	4.063	1.92	24.571	3.00	4.039	4.08	2.268
.92	4.506	2.00	17.735	3.08	3.858		

1.00 5.068 | 2.08 13.772 | 3.17 3.492 |
1.08 5.795 | 2.17 11.219 | 3.25 3.309 |

003:0003-----

PROPOSED SITE

*AREA 201 -> PROPOSED LONGWOODS ROAD INDUSTRIAL SUBDIVISION

1 CALIB STANDBYD Area (ha) = 6.65 Dir. Conn. (%) = 50.00
1 01:201 DI= 5.00 Total Imp (%) = 60.00

Surface Area (ha) = 3.99 IMPERVIOUS PERVIOUS (i)

Dep. Storage (mm) = 2.00

Average Slope (%) = 1.00

Length (m) = 80.00

Mannings n = .035

Max. eff. Inten. (mm/hr) = 134.79

Storage Coeff. (min) = 5.00

Unit Hyd. Tpeak (min) = 2.16 (ii)

Unit Hyd. peak (cms) = 5.00

PEAK FLOW (cms) = 1.16

TIME TO PEAK (hrs) = 1.67

RUNOFF VOLUME (mm) = 44.48

TOTAL RAINFALL (mm) = 46.48

RUNOFF COEFFICIENT = .96

*** WARNING: Storage Coefficient is smaller than DT!

Use a smaller DT or a larger area.

(i) CN* = 74.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DI) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

003:0004-----

1 COMPUTE VOLUME

1 ID:01 (201)

START CONTROLLING AT DISCHARGE TIME

INFLOW HYD. PEAKS AT .000 (hrs)

STOP CONTROLLING AT 1.371 1.667

REQUIRED STORAGE VOLUME (ha.m.) = .1896

TOTAL HYDROGRAPH VOLUME (ha.m.) = .2006

% OF HYDROGRAPH TO STORE = 94.5531

NOTE: Storage was computed to reduce the Inflow

peak to .017 (cms).

003:0005-----

003:0002-----

END OF RUN : 3

1 START Project dir.: Q:\45013\200\SWMMHYMO\60%T1M-1\

1 ZERO = .00 hrs on Rainfall dir.: Q:\45013\200\SWMMHYMO\60%T1M-1\

1 METOUT= 2 (output = METRIC)

1 NRUN = 004

1 NSTORM= 1

1 # 1=10YR.HYT

004:0002-----

Project Name: [10919 LONGWOODS ROAD INDUSTRIAL SUBDIVISION] Project Numbe

Date : MARCH 2020

Modeler : [MEM]

Company : MTE CONSULTANTS INC.

License # :

004:0002-----

1 READ STORM Filename: 10YR Chicago 4-hr duration

1 Ptotal= 54.60 mm Comments: 10YR Chicago 4-hr duration

TIME RAIN TIME RAIN TIME RAIN

hrs mm/hr hrs mm/hr hrs mm/hr

.08 2.343 1.17 7.804 2.25 11.115

.17 2.473 1.25 9.514 2.33 9.514

.25 2.618 1.33 12.124 2.42 8.304

.33 2.783 1.42 16.521 2.50 7.361

.42 2.971 1.50 25.144 2.58 6.607

.50 3.187 1.58 47.356 2.67 5.991

.58 3.439 1.67 156.469 2.75 5.481

.67 3.735 1.75 93.617 2.83 5.050

.75 4.089 1.83 47.356 2.92 4.682

.83 4.518 1.92 30.071 3.00 4.365

.92 5.050 2.00 21.499 3.08 4.089

1.00 5.725 2.08 16.521 3.17 3.846

1.08 6.607 2.17 13.321 3.25 3.631

TIME RAIN

hrs mm/hr

3.33 3.439

3.42 3.267

3.50 3.112

3.58 2.971

3.67 2.843

3.75 2.726

3.83 2.618

3.92 2.519

4.00 2.428

4.08 2.343

4.17 2.266

4.25 2.189

4.33 2.112

4.42 2.035

4.50 1.958

4.58 1.881

4.67 1.804

4.75 1.727

4.83 1.650

4.92 1.573

5.00 1.496

5.08 1.419

5.17 1.342

5.25 1.265

5.33 1.188

5.42 1.111

5.50 1.035

5.58 0.958

5.67 0.881

5.75 0.804

5.83 0.727

5.92 0.650

6.00 0.573

6.08 0.496

6.17 0.419

6.25 0.342

6.33 0.265

6.42 0.188

6.50 0.111

6.58 0.035

6.67 0.000

6.75 0.000

6.83 0.000

6.92 0.000

7.00 0.000

7.08 0.000

7.17 0.000

7.25 0.000

7.33 0.000

7.42 0.000

7.50 0.000

7.58 0.000

7.67 0.000

7.75 0.000

7.83 0.000

7.92 0.000

8.00 0.000

8.08 0.000

8.17 0.000

8.25 0.000

8.33 0.000

8.42 0.000

8.50 0.000

8.58 0.000

8.67 0.000

8.75 0.000

8.83 0.000

8.92 0.000

9.00 0.000

9.08 0.000

9.17 0.000

9.25 0.000

9.33 0.000

9.42 0.000

9.50 0.000

9.58 0.000

9.67 0.000

9.75 0.000

9.83 0.000

9.92 0.000

10.00 0.000

10.08 0.000

10.17 0.000

10.25 0.000

10.33 0.000

10.42 0.000

10.50 0.000

10.58 0.000

10.67 0.000

10.75 0.000

10.83 0.000

10.92 0.000

11.00 0.000

11.08 0.000

11.17 0.000

11.25 0.000

11.33 0.000

11.42 0.000

11.50 0.000

11.58 0.000

11.67 0.000

11.75 0.000

11.83 0.000

11.92 0.000

12.00 0.000

12.08 0.000

12.17 0.000

12.25 0.000

12.33 0.000

12.42 0.000

12.50 0.000

12.58 0.000

12.67 0.000

12.75 0.000

12.83 0.000

12.92 0.000

13.00 0.000

13.08 0.000

13.17 0.000

13.25 0.000

13.33 0.000

13.42 0.000

13.50 0.000

13.58 0.000

13.67 0.000

13.75 0.000

13.83 0.000

13.92 0.000

14.00 0.000

14.08 0.000

14.17 0.000

14.25 0.000

14.33 0.000

14.42 0.000

14.50 0.000

14.58 0.000

14.67 0.000

14.75 0.000

14.83 0.000

14.92 0.000

15.00 0.000

15.08 0.000

15.17 0.000

15.25 0.000

15.33 0.000

15.42 0.000

15.50 0.000

15.58 0.000

15.67 0.000

15.75 0.000

15.83 0.000

15.92 0.000

16.00 0.000

16.08 0.000

16.17 0.000

16.25 0.000

16.33 0.000

16.42 0.000

16.50 0.000

16.58 0.000

16.67 0.000

16.75 0.000

16.83 0.000

16.92 0.000

17.00 0.000

17.08 0.000

17.17 0.000

17.25 0.000

17

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
004:0004-----
```

	DISCHARGE (cms)	TIME (hrs)
START CONTROLLING AT	.900	.667
INFLOW HYD. PEAKS AT	1.866	1.667
STOP CONTROLLING AT	.017	4.140

REQUIRED STORAGE VOLUME (ha.m.) =	.2338
TOTAL HYDROGRAPH VOLUME (ha.m.) =	.2447
% OF HYDROGRAPH TO STORE	= 95.5422

NOTE: Storage was computed to reduce the Inflow peak to .017 (cms).

```
004-0005-----
-----
004-0002-----
-----
004-0002-----
-----
004-0002-----
** END OF RUN : 4
```

```

[ START | Project dir.: Q:\45013\200\SMWHYMO\60\T1M-1\
          Rainfall dir.: Q:\45013\200\SMWHYMO\60\T1M-1\

          TZERO = 00 hrs on 0
          METOUT= 2 (output = METRIC)
          NRUN = 005
          NSTORM= 1

```

```

005:0002-----
# # Project Name: [10919 LONGWOODS ROAD INDUSTRIAL SUBDIVISION] Project Number
# # Date : MARCH 2020
# # Modeler : [MFM]
# # Company : MTE CONSULTANTS INC.
# # License # :

```

READ STORM	Filename: 25YR Chicago	4-hr duration
Ptotal= 64.33 mm	Comments: 25YR Chicago	4-hr duration

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	2.569	1.17	9.034	2.25	13.050	3.33	3.036	4.38	2.569
1.17	2.718	1.25	11.103	2.33	11.103	3.42	3.436	4.48	2.569
2.25	2.865	1.33	14.280	2.42	9.837	3.50	3.436	4.58	2.569
3.33	3.012	1.42	15.956	2.50	6.959	3.59	3.436	4.68	2.569
4.42	3.043	1.50	17.277	2.59	6.823	3.67	3.444	4.78	2.569
5.50	3.254	1.58	19.273	2.67	6.823	3.75	3.444	4.88	2.569
6.58	3.836	1.67	14.548	2.75	6.823	3.83	3.444	4.98	2.569
7.67	4.183	1.75	12.372	2.83	5.734	3.92	3.444	5.08	2.569
8.75	4.598	1.83	17.277	2.92	5.298	4.00	2.569	5.18	2.569
9.83	5.104	1.92	36.251	3.00	4.923	4.08	2.569	5.28	2.569
10.92	5.734	2.00	25.756	3.08	4.923	4.18	2.569	5.38	2.569
12.00	6.537	2.08	19.656	3.17	4.312	4.28	2.569	5.48	2.569

1.08	7.593	2.17	15.741	3.25	4.060
------	-------	------	--------	------	-------

00:00:03-----PROPOSED SITE-----

*AREA 201 -> PROPOSED LONGWOODS ROAD INDUSTRIAL SUBDIVISION

	Area	(ha)	
CALIB STANDHYD	6.65		
01:201	DT= 5.00	Total Imp%	60.00 Dir. Conn. (%)= 50.00

	INTERVIEW	PREVIOUS (1)
Surface Area	(ha) =	2.66
Dep. Storage	(mm) =	3.99
Average Slope	(%) =	2.00
Length	(m) =	5.00
Mannings n	=	1.00
		2.50
		80.00
		.015
		.035

Max. eff. inten. (mm/hr) =	194.48	8.26
over (min)	5.00	5.00
Storage Coeff. (in)	1.91	1.12
Unit Hyd. peak (in)	5.00	5.00
Unit Hyd. peak (cms)	3.2	.21

	1.62	.51	2.062 (41)
PEAK FLOW (cms)	1.62	.51	2.062 (41)
TIME TO PEAK (hrs)	1.67	1.75	1.667
RUNOFF VOLUME (mm)	62.33	27.63	44.577
TOTAL RAINFALL (mm)	64.33	64.33	64.326
RUNOFF COEFFICIENT	.97	.43	.699

*** WARNING: Storage Coefficient is smaller than DT!
 Use a smaller DT or a larger area.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

[illegible]

DISCHARGE		TIME
(cms)		(hrs)
1	COMPUTE VOLUME	.000
2	ID:01 (201)	*.583
3	START CONTROLLING AT	1.567
4	INFLOW HYD. PEAKS AT	2.062
5	STOP CONTROLLING AT	2.062
6		.017
7		4.144

REQUIRED STORAGE VOLUME (ha.m.)	=	.2880
TOTAL HYDROGRAPH VOLUME (ha.m.)	=	.2991
% OF HYDROGRAPH TO STORE	=	96.2738

NOTE: Storage was computed to reduce the Inflow peak to .017 (cms).

```
005:0005-----
005:0002-----
005:0002-----
005:0002-----
005:0002-----
005:0002-----
** END OF RUN : 5
```

START Project dir.: Q:\45013\200\SWHMYMO\60%TIM-1\
Rainfall dir.: Q:\45013\200\SWHMYMO\60%TIM-1\
TZERO = 00 hrs on 0
METOUT = 2 (output = METRIC)
NSTORM = 006
NSTORM = 1
1=50YR.HYT

006:0002-----
COMPUTE VOLUME
ID:01 (201)
Project Name: [10919 LONGWOODS ROAD INDUSTRIAL SUBDIVISION] Project Numbe
Date : MARCH 2020
Modeler : [MEM]
Company : MTE CONSULTANTS INC.
License # :
006:0002-----

006:0002-----
READ STORM
Filename: 50YR Chicago 4-hr duration
Ptotal= 71.91
Comments: 50YR Chicago 4-hr duration

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs
08	2.864	1.17	10.121	2.25	14.636	3.33	4.283
17	3.030	1.25	12.446	2.33	12.446	3.42	4.059
25	3.218	1.33	16.018	2.42	10.799	3.50	3.856
33	3.430	1.42	22.060	2.50	9.519	3.58	3.674
42	3.674	1.50	33.926	2.58	8.501	3.67	3.508
50	3.955	1.58	44.215	2.67	7.673	3.75	3.356
58	4.283	1.67	205.106	2.75	6.988	3.83	3.218
67	4.672	1.75	125.487	2.83	6.413	3.92	3.090
75	5.137	1.83	64.215	2.92	5.924	4.00	2.973
83	5.706	1.92	40.686	3.00	5.503	4.08	2.864
92	6.413	2.00	28.913	3.08	5.137		
1.00	7.315	2.08	22.060	3.17	4.817		
1.08	8.501	2.17	17.661	3.25	4.535		

006:0003-----
PROPOSED SITE
*AREA 201 -> PROPOSED LONGWOODS ROAD INDUSTRIAL SUBDIVISION
CALIB STANDHYD
01:201 DT= 5.00 Area (ha)= 6.65 Total Imp(%)= 60.00 Dir. Conn.(%)= 50.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 3.99 2.66
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 1.00 2.50
Length (m)= 80.00 80.00
Mannings n = .015 .035
Max. eff. Inten. (mm/hr)= 205.11 108.38
Storage Coeff. (min)= 5.00 5.00
Unit Hyd. Tpeak (min)= 1.83 (ii) 4.80 (ii)
Unit Hyd. peak (cms)= .32 .22
PEAK FLOW (cms)= 1.81 .62
TIME TO PEAK (hrs)= 1.67 1.75
RUNOFF VOLUME (mm)= 69.91 33.10
TOTAL RAINFALL (mm)= 71.91 51.505
RUNOFF COEFFICIENT = .97 .46
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.

- (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

006:0004-----
COMPUTE VOLUME
ID:01 (201)
Project Name: [10919 LONGWOODS ROAD INDUSTRIAL SUBDIVISION] Project Numbe
Date : MARCH 2020
Modeler : [MEM]
Company : MTE CONSULTANTS INC.
License # :
006:0004-----

006:0004-----
READ STORM
Filename: 100YR Chicago 4-hr duration
Ptotal= 79.25
Comments: 100YR Chicago 4-hr duration

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs
08	2.864	1.17	10.121	2.25	14.636	3.33	4.283
17	3.030	1.25	12.446	2.33	12.446	3.42	4.059
25	3.218	1.33	16.018	2.42	10.799	3.50	3.856
33	3.430	1.42	22.060	2.50	9.519	3.58	3.674
42	3.674	1.50	33.926	2.58	8.501	3.67	3.508
50	3.955	1.58	44.215	2.67	7.673	3.75	3.356
58	4.283	1.67	205.106	2.75	6.988	3.83	3.218
67	4.672	1.75	125.487	2.83	6.413	3.92	3.090
75	5.137	1.83	64.215	2.92	5.924	4.00	2.973
83	5.706	1.92	40.686	3.00	5.503	4.08	2.864
92	6.413	2.00	28.913	3.08	5.137		
1.00	7.315	2.08	22.060	3.17	4.817		
1.08	8.501	2.17	17.661	3.25	4.535		

007:0002-----
START Project dir.: Q:\45013\200\SWHMYMO\60%TIM-1\
Rainfall dir.: Q:\45013\200\SWHMYMO\60%TIM-1\
TZERO = 00 hrs on 0
METOUT = 2 (output = METRIC)
NSTORM = 007
NSTORM = 1
1=100YR.HYT
007:0002-----
COMPUTE VOLUME
ID:01 (201)
Project Name: [10919 LONGWOODS ROAD INDUSTRIAL SUBDIVISION] Project Numbe
Date : MARCH 2020
Modeler : [MEM]
Company : MTE CONSULTANTS INC.
License # :
007:0002-----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs
08	2.864	1.17	10.121	2.25	14.636	3.33	4.283
17	3.030	1.25	12.446	2.33	12.446	3.42	4.059
25	3.218	1.33	16.018	2.42	10.799	3.50	3.856
33	3.430	1.42	22.060	2.50	9.519	3.58	3.674
42	3.674	1.50	33.926	2.58	8.501	3.67	3.508
50	3.955	1.58	44.215	2.67	7.673	3.75	3.356
58	4.283	1.67	205.106	2.75	6.988	3.83	3.218
67	4.672	1.75	125.487	2.83	6.413	3.92	3.090
75	5.137	1.83	64.215	2.92	5.924	4.00	2.973
83	5.706	1.92	40.686	3.00	5.503	4.08	2.864
92	6.413	2.00	28.913	3.08	5.137		
1.00	7.315	2.08	22.060	3.17	4.817		
1.08	8.501	2.17	17.661	3.25	4.535		

```

-50 4.230 1.58 71.789 2.67 9.347 3.75 3.574
-58 4.590 1.67 25.339 2.56 8.347 3.83 3.423
-67 5.018 1.75 19.138 2.83 6.584 3.92 3.284
-75 5.532 1.83 11.788 3.22 6.945 4.00 3.156
-83 6.166 1.92 45.783 3.00 5.936 4.08 3.037
-92 6.949 2.00 32.356 3.00 5.936 4.08 3.037
1.00 7.948 2.08 24.269 3.97 5.352 4.08 3.037
1.08 9.270 2.17 19.354 3.25 4.867 4.08 3.037

```

007:0003

PROPOSED SITE

*AREA 201 -> PROPOSED LONGWOODS ROAD INDUSTRIAL SUBDIVISION

```

| CALIB STANDBYD | Area (ha) = 6.65 | Dir. Conn. (%) = 50.00
| 01:201 DT= 5.00 | Total Imp (%) = 60.00

```

```

| IMPERVIOUS | Pervious (i)
| Surface Area (ha) = 3.99 |
| Dep. Storage (mm) = 2.66 |
| Average Slope (%) = 1.00 |
| Length (m) = 80.00 |
| Mannings n = .035 |

```

Max. eff. Inten. (mm/hr) = 225.19

Storage Coeff. (min) = 1.76 (ii)

Unit Hyd. Peak (cms) = 5.00

Unit Hyd. Peak (cms) = .32

PEAK FLOW (cms) = 74

TIME TO PEAK (hrs) = 1.67

RUNOFF VOLUME (mm) = 77.25

TOTAL RAINFALL (mm) = 79.25

RUNOFF COEFFICIENT = .97

*** WARNING: Storage Coefficient is smaller than DT!

Use a smaller DT or a larger area.

(i) CN* = 74.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TOTALS

2.675 (iii)

57.953

79.251

.731

007:0004

COMPUTE VOLUME

ID:01 (201)

START CONTROLLING AT

INFLOW HYD. PEAKS AT

STOP CONTROLLING AT

REQUIRED STORAGE VOLUME (ha.m.) = 3739

TOTAL HYDROGRAPH VOLUME (ha.m.) = 3853

% OF HYDROGRAPH TO STORE

NOTE: Storage was computed to reduce the Inflow

peak to .017 (cms).

007:0005

007:0002

007:0002

007:0002

007:0002

007:0002

007:0002

007:0002

007:0002

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007:0002

007:0002

007:0002

007:0002

007:0002

007:0002

007:0002

3.08	7.99	9.17	12.284	15.25	1.397	21.33	.685
3.17	8.09	9.26	15.202	15.25	1.397	21.42	.680
3.25	8.19	9.33	19.711	15.2	1.397	21.50	.676
3.33	8.29	9.42	17.388	15.2	1.397	21.58	.671
3.42	8.39	9.50	42.892	15.0	1.397	21.67	.667
3.50	8.50	9.58	81.712	15.0	1.397	21.75	.662
3.58	8.61	9.67	265.689	15.0	1.397	21.83	.658
3.67	8.73	9.75	11.112	15.0	1.397	21.92	.654
3.75	8.84	9.83	11.112	15.0	1.397	22.00	.650
3.83	8.94	9.92	21.588	15.0	1.397	22.08	.646
3.92	9.09	10.00	21.588	15.0	1.397	22.17	.642
4.00	9.22	10.08	21.588	15.0	1.397	22.25	.638
4.08	9.32	10.17	21.588	15.0	1.397	22.33	.634
4.17	9.48	10.25	17.323	15.0	1.397	22.42	.630
4.25	9.63	10.33	15.323	15.0	1.397	22.50	.626
4.33	9.77	10.42	13.323	15.0	1.397	22.58	.622
4.42	9.92	10.50	11.323	15.0	1.397	22.67	.618
4.50	10.07	10.58	10.323	15.0	1.397	22.75	.615
4.58	10.23	10.67	9.323	15.0	1.397	22.83	.611
4.67	10.40	10.75	8.323	15.0	1.397	22.92	.607
4.75	10.57	10.83	7.323	15.0	1.397	23.00	.604
4.83	10.74	10.92	6.323	15.0	1.397	23.08	.600
4.92	10.93	11.00	5.323	15.0	1.397	23.17	.597
5.00	11.12	11.08	4.323	15.0	1.397	23.25	.593
5.08	11.32	11.17	3.323	15.0	1.397	23.33	.590
5.17	11.52	11.25	2.323	15.0	1.397	23.42	.587
5.25	11.73	11.33	1.323	15.0	1.397	23.50	.583
5.33	11.93	11.42	4.782	15.0	1.397	23.58	.580
5.42	12.18	11.50	4.580	15.0	1.397	23.67	.577
5.50	12.41	11.58	4.318	15.0	1.397	23.75	.574
5.58	12.66	11.67	4.117	15.0	1.397	23.83	.571
5.67	12.92	11.75	3.934	15.0	1.397	23.92	.567
5.75	13.19	11.83	3.766	15.0	1.397	24.00	.564
5.83	13.47	11.92	3.613	15.0	1.397	24.08	.561
5.92	13.77	12.00	3.471	15.0	1.397		
6.00	14.08	12.08	3.340	15.0	1.397		
6.08	14.40	12.17	3.219	15.25	1.397		

008:0003-----FINISH

PROPOSED SITE

AREA 201 -> PROPOSED LONGWOODS ROAD INDUSTRIAL SUBDIVISION

01:201	DT= 5.00	Area	(ha)=	6.65	Dir. Conn. (\$)=	50.00
Surface Area	(ha)=	IMPERVIOUS	PERVIOUS (i)			
Dep. Storage	(mm)=	3.99	2.66			
Average Slope	(%)=	2.00	5.00			
Length	(m)=	1.00	2.50			
Mannings n		80.00	80.00			
Max. eff. Inten. (mm/hr)=		.015	.035			
Storage Coeff. (min)=		265.69	192.02			
Unit Hyd. Peak (cms)=		5.00	5.00			
Unit Hyd. Peak (cms)=		1.65 (ii)	4.02 (ii)			
PEAK FLOW (cms)=		.32	.24			
TIME TO PEAK (hrs)=		2.37	1.09			
RUNOFF VOLUME (mm)=		9.67	9.67			
TOTAL RAINFALL (mm)=		111.56	66.33			
RUNOFF COEFFICIENT		113.56	113.56			
*** WARNING: Storage Coefficient is smaller than DT!		.98	.58			
Use a smaller DT or a larger area.						
TOTALS						
						3.461 (iii)
						9.667
						88.945
						113.558
						.783

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

008:0004-----

ID:01 (201)	DISCHARGE (cms)	TIME (hrs)
START CONTROLLING AT	.000	3.000
INFLOW HYD. PEAKS AT	3.461	9.667
STOP CONTROLLING AT	.017	17.513
REQUIRED STORAGE VOLUME (ha.m.)		.5172
TOTAL HYDROGRAPH VOLUME (ha.m.)		.5915
% OF HYDROGRAPH TO STORE		87.4426

NOTE: Storage was computed to reduce the Inflow peak to .017 (cms).

008:0005-----

008:0002-----

008:0002-----

008:0002-----

008:0002-----

008:0002-----

FINISH

WARNINGS / ERRORS / NOTES

001:0003 CALIB STANDHYD

- *** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
*** WARNING: Storage Coefficient is smaller than DT!
Use a smaller DT or a larger area.
Simulation ended on 2020-06-25 at 20:06:41