



July 14, 2021
File: LD-00203

VIA EMAIL

The Municipality of Middlesex Centre
10227 Ilderton Road
Ilderton, ON N0M 2A0

Attention: Mr. Jake Deridder
Development Review Coordinator, Department Public Works & Engineering

Dear Jake:

Reference: MN 6, 10, 14 Elmhurst Street, Kilworth
Sweid Holdings Inc.
Stormwater Servicing Brief

Introduction

LDS Consultants Inc. (LDS) has been retained by Sweid Holdings Inc. to prepare a strategy to provide stormwater management infrastructure servicing for the proposed development of the property located at MN 6, 10 and 14 Elmhurst Street in Town of Kilworth as illustrated in **Figure 1**. The subject site comprises open green space, and is approximately 2.02 hectares in area. The site is bounded by Elmhurst Street to the east, and existing residential lots with wooded areas to the north, south and west.

Background Information

The servicing strategy presented herein was developed using the following information;

- Geotechnical Investigation, prepared by LDS Consultants Inc., dated October 8th, 2019; and,
- Middlesex Centre Infrastructure Design Standards, dated January 2018;

Existing Condition

The site currently consists of three existing residential lots. These lots are generally comprised of open grassed area, multiple buildings, and some trees. The site drains west towards the rear of the lots. External flows enter the site through the north-most lot line; these external areas consist mainly of woodlot. The existing catchment areas are described below. The site was modelling in SWMHYMO to calculate existing peak flows which are summarized in **Table 1** and the catchments are illustrated in **Figure 2**. Model documentation is attached.

Catchment 101 – This catchment consists of the existing greenspace, has some trees and three existing residential dwellings and few sheds. This catchment drains overland west towards existing residential lots.

Catchment EX-1 – This catchment represents a wooded area to the northwest of the site. These area drains southeast into the existing greenspace.

Table 1 - Pre-Development Runoff Peak Flows

Storm Event	Peak Flow (m³/s)
2-year	0.057
5-year	0.060
10-year	0.088
25-year	0.126
50-year	0.154
100-year	0.190
250-year	0.247

A geotechnical investigation was conducted by LDS to determine existing soil conditions at the subject site. The predominate soil on site is a sandy loam. For modelling purposes, it will be classified as Hydrologic Soil Group A. Five of the six boreholes were observed as being dry while a groundwater level was observed in one test pit at a depth of 4.27 meters.

Proposed Condition

It is proposed to develop the site as a mixture residential development consisting of 28 single detached homes and 28 town houses. The proposed development will include paved areas as well as some open space grassed areas. All storm runoff occurring onsite will be conveyed to the internal storm sewer system, and be allowed to infiltrate through sub terrain, stone infiltration galleries. The proposed catchments are described below and are illustrated in **Figure 3**. The proposed condition was modelled using SWMHYMO and the model results are attached to this letter report.

Catchment 201 – This catchment consists of backyards of single dwelling units 1 - 11. Minor and major flows will be conveyed to a 120 m long linear stone infiltration gallery adjacent to the lot line. This catchment accepts external flows from Catchment EX-1.

Catchment 202 – This catchment consists of backyards of single dwelling units 12 and 13. Minor and major flows will be conveyed to open bottom catch basin within lot 12. This catchment accepts external flows from Catchment EX-2.

Catchment 203 – This catchment consists of backyards of single dwelling units 14 - 16. Minor and major flows will be conveyed to a 50 m long linear stone infiltration gallery adjacent to the lot line.

Catchment 204 – This catchment consists of backyards of single dwelling units 17 - 28. Minor and major flows will be conveyed to a 120 m long linear stone infiltration gallery adjacent to the lot line.

Catchment 205 – This catchment consists of the internal street network, green space and fifty-six (56) dwellings. Minor and major flows will be conveyed by the storm sewer network to the stone infiltration gallery.

Catchment 206 – This catchment consists of a small portion of rear and side yard in the north portion of the site. Minor and major flows will be conveyed to the Elmhurst Street ditch. This catchment accepts external flows from Catchment EX-3.

Catchment EX-1 – This catchment represents a wooded area to the northwest of the site. This area drains southeast onto the subject site. Minor and major flows will be conveyed by the storm sewer network to the stone infiltration gallery within Catchment 201.

Catchment EX-2 – This catchment represents a wooded area to the northwest of the site. This area drains southeast onto the subject site. Minor and major flows will be conveyed by the storm sewer network to the stone infiltration gallery within Catchment 202.

Catchment EX-3 – This catchment represents a wooded area to the north of the site. This area drains southeast onto the subject site. Minor and major flows will be conveyed through the subject site to the Elmhurst Street ditch.

Quantity Control

Quantity control is achieved through the combination of surface ponding, a perforated/oversized storm sewer network and an infiltration gallery underneath the internal road network. There is no outlet for this site. As such, all flow will be contained within the site boundaries to avoid adverse effects to the surrounding environment. Runoff from the backyards of the single detached homes will flow to open bottom catchbasins located in the rear yards. Runoff from the fronts of houses and town homes will flow to the internal asphalt surface. The asphalt surface will be conveyed by catchbasins and the internal storm sewer, which captures all flows on site. This system is oversized and utilizes perforated pipes to infiltrate the collected stormwater into the underlying stone bed. This infiltration gallery will store and then infiltrate all flows on site. The information used to model the infiltration gallery is outlined in **Table 3** and the footprint of this infiltration gallery is depicted in **Figure 3**, as well. There is a catchbasin manhole located in the depressed greenspace between the two parking areas in the middle of site. This area will serve as a surface storage area. The culmination of these controls is summarized in **Table 2**. Supporting model documentation is attached.

Table 2 – Infiltration Gallery Modelling Parameters

Attribute	201	202	203	204	205
Surface Area (m ²)	120	0.87	50	120	3495
Void Ratio	0.40	0.40	0.40	0.40	0.40
Depth of Stone Bed (m)	1.0	1.8	1.0	1.0	0.5
Available Void Space (m ³)	48	2	20	48	699
Surface Ponding Volume (m ³)	60	5	20	60	62
Total Storage Volume (m ³)	108	7	40	108	761
Infiltration Rate (mm/hr)	25	25	25	25	25
Model Flowrate (m ³ /s)	0.00083	0.00035	0.00035	0.00083	0.0235
2-year Volume Used (m ³)	41	7	7	23	118
250-year Volume Used (m ³)	108	7	37	92	576

As shown, the total post development peak flow rates are less than the pre-development condition as the entire volume of the 250-year storm is infiltrated without overflowing the storage system. Approximately 579 m³ of underground storage in the main infiltration gallery is utilized in the 250-year storm event. As the primary storage facility for the subject site, this infiltration gallery has an additional 185 m³ of subsurface and surface storage for greater storm events. Should the onsite storage be overwhelmed, the ultimate overland outlet for the subject site is Elmhurst Street.

Erosion and Sediment Control

This section describes the Erosion and Sediment Control Plan that will be implemented prior to, during and immediately after construction to reduce the possibility of sediment being conveyed from the proposed construction site.

Types of Selected Erosion / Sediment Control Methods

The details and locations of the proposed temporary and longer-term erosion and sediment control measures will be identified prior to final approval. The construction drawings, once complete, will form a part of the sediment and erosion control plan. Proposed erosion and sediment control measures include the following:

- Silt fence and robust siltation barrier will be installed along the boundary of the subject site.
- All disturbed areas where work will not take place for a period of 30 days or more will be stabilized in accordance with OPSS 572.
- Street sweeping will be performed as necessary to remove soil deposited on adjacent right-of ways by construction traffic.

The proposed temporary erosion and sediment control measures have been selected based on the site's susceptibility to erosion, sensitivity of the downstream environment, site slopes, and total drainage area. The proposed measures should provide adequate erosion and sediment control for the proposed project without the need for additional measures. However, the site will be monitored during construction, and additional measures will be added, if required.

Installation of Erosion Control Measures

Proposed erosion and sediment control measures are summarized in **Table 4**.

Table 3 – Erosion and Sediment Control Sequencing

Stage	Erosion and Sediment Control Measures
Pre-Construction	Create contact list for emergency contingency plan operations.
	Install silt fence around the proposed work limits, as appropriate.
	Install robust siltation barrier.
	Preparation of a Construction Dewatering Discharge Plan, including discharge location, temporary storage locations and identifying measures to reduce suspended solids or other treatment, if required

Construction	Monitor water quality (turbidity) for construction dewatering discharge water discharged at surface
	Regular inspection of erosion and sediment control measures to confirm they are effective and operating as intended.
	Monitor weather reports for significant precipitation events for contingency planning.
	Install filter cloth in on-site catchbasins.
	Perform street sweeping as necessary to remove accumulated sediment from the adjacent right-of-way.
	Complete final paving.
	Complete final landscaping and vegetation plantings.
Post-Construction	Remove robust siltation barrier.
	Remove silt fence from the proposed work limits
	Remove filter cloth from on-site catchbasins.
	Remove construction fence from the proposed work limits.

The proposed erosion and sediment control measures have been designed according to the site slopes, the site drainage area, and the risks and consequences of failure. Based on these factors, additional measures will likely not be required.

However, the site will be monitored during construction and additional measures (i.e. additional rows of silt fence) may be installed at the discretion of the Contract Administrator. Although this not an exhaustive list, inspections are expected to include checks on siltation barrier installations to confirm that it is properly installed and secured, including inspection for evidence of damage or tears, and overtopping or undermining; checking condition of surface water ponding areas and storm drain inlets, and documenting areas where seeding / sodding / mulching is implemented to re-establish vegetative cover.

The triggers for the installation of enhanced erosion and sediment control measures would include breaching of the proposed erosion and sediment control measures, and / or re-evaluation based on site conditions during construction. In any event, site conditions and the proposed erosion and sediment control measures will be monitored on a regular basis, as described below.

Inspection Requirements

In order to monitor the effectiveness of the erosion and sediment control measures during site grading and site servicing work, frequent inspections will be required. The following minimum inspection intervals are recommended:

- The Contractor and Contract Administrator shall monitor weather reports on a daily basis, and record daily temperatures and rainfall. When rainfall is anticipated the Contractor and the Contract Administrator shall inspect the erosion control works immediately before and immediately after rainfall event and snowmelt event (timing for inspections before are based on predicted weather forecasts);
- Daily during extended or significant precipitation (i.e. rainfall amounts that exceed 25 millimetres) or during significant snowmelt periods;
- Daily during any construction activity that would potentially yield significant run-off volumes or otherwise impact the quality of the run-off leaving the site;
- Daily while deficiencies are present which fail to contain, filter or otherwise treat run-off, or contribute to sediment loading in surface water;
- Weekly during dry periods while construction activity is occurring at the site. The Contractor and the Contract Administrator shall inspect the erosion control measures the day before the last business day of the week (typically Thursday) to allow for any work to be completed on damaged erosion control works prior to the weekend.; and,
- Monthly during inactive periods (> 30 days).

The Contract Administrator will document all inspection activities in weekly erosion and sediment control inspection reports.

The Contractor shall be responsible for constructing and maintaining all erosion and sediment control measures. This shall include, but not be limited to: maintaining fencing, and removing accumulated sediment. Temporary erosion and sediment control measures will not be removed until the areas they serve are restored and stable. Removal of the erosion and sediment control measures will be the responsibility of the builder after sod has rooted on the site.

Contingency Plan

The purpose of the contingency plan is to help minimize the risk or consequence of failure of the erosion and sediment control works. Failure could result from insufficient measures, lack of maintenance, or severe weather conditions. The contingency plan includes two areas of consideration:

- Procedures that will be followed where a failure has occurred; and
- Contingency measures that will be implemented where there is potential for failure.

The Contractor shall be responsible for following the contingency plan, and will prepare the following items:

- The Contractor will maintain a contact list for emergency situations.
- Workers shall be on call for emergency situations for all aspects of the emergency from design to installation of sediment and erosion control measures. Any associated health and safety issues are the responsibility of Contractor.
- Sediment and erosion control measures such as erosion control blanket, straw bales and stakes, sandbags, and silt fence shall be made available for emergency installation.
- Gas powered pumps, appropriately sized hoses, filtration hose socks, and filter cloth shall be made available for emergency dewatering.
- Heavy equipment shall be on standby for emergency works.
- A supplemental contact list for any further required equipment or materials shall be prepared and made available for emergency use.

Monitoring

As noted previously, regular monitoring of the erosion and sediment control measures at the site makes up an integral part of the contingency plan, by providing an early indication should any environmental control measures (such as sediment and erosion control measures) or practices fail to achieve prescribed standards. Recommended inspection intervals were discussed previously.

In the event that monitoring identifies a high potential for failure, steps shall be immediately taken to reduce the risk. These measures may include repair to existing measures, modification of existing measures, and the addition of new measures.

If unforeseen events cause the strategies set out in the contingency plan to be insufficient or inappropriate to meet the objective of containing sediment within the working limits, the Contractor, either independently or as directed by the Contract Administrator, will respond in a timely manner with all reasonable measures to prevent, counteract, or remedy any effects on aquatic habitat, and human interest (i.e. public safety, property value).

Updates to the Erosion and Sediment Control Plan may be required to reflect changes at the site, during various stages of construction. The municipality will be circulated with updated plans, to ensure that they have the most current plans available for review and consultation, if needed.

Severe Weather Anticipated

In cases where the weather forecast indicates that significant rainfall is expected within a 24-hour period, the Contractor shall immediately complete the following:

- Inspect existing erosion and sediment control measures to confirm that they are secure and in good working order;
- Review site conditions to identify and protect areas of exposed soil that could be susceptible to surface erosion; and,
- Monitor all measures during the rainfall event, and where a potential for failure is identified, take corrective action.

The Contract Administrator shall document the status of the above-listed steps.

Responding to Failures

The Contractor will cease all construction related work and focus on erosion and sediment control as required to effectively stabilize the site where a failure has occurred or is imminent. The work shall be completed to the satisfaction of the Contract Administrator and any regulatory agencies having jurisdiction.

Any unexpected discharge of silt, sediment, or other deleterious substance shall be reported to the Municipality of Middlesex Centre within a period of 2 hours. The contractor is responsible to advise the contract administrator, and to report the incident to the Spills Action Centre in a timely manner. Depending on the type of incident, water sampling and quality testing may be warranted to document the extent of the impact. Scoping for the required testing will depend on the incident report.

If significant long-term damage to aquatic habitat or property is suspected, a restoration plan will be developed by the Owner's Engineer. Consultation with an ecologist and / or biologist may be required to confirm that the remedial measures are appropriate. Development of the initial restoration plan will begin within 24 hours of the discovery of sediment discharge, and will be implemented as soon as possible following consultation and approval from the MECP, Upper Thames Conservation Authority (UTRCA), and Municipality of Middlesex Centre. The plan will address:

- Removal and disposal of sediment deposited outside of the work limits; and
- Restoration of any areas disturbed through deposition or removal.

Reporting Schedule

The Contract Administrator shall prepare weekly erosion and sediment control monitoring reports/summaries for the duration of site grading and site servicing, and submit them to the Municipality of Middlesex Centre by April 1, July 1, and November 1 of each year until all works and services included in the plan are assumed.

The monitoring reports should document the status of the ESC Plan, any repairs, rainfall or pumping that has occurred since the last report, and any risks of failure that may be present.

Additionally, any failure of the erosion and sediment control measures shall be reported as described in the contingency plan.

Construction Dewatering Requirements

Based on the findings of the Geotechnical Investigation prepared by LDS Consultants, no shallow groundwater conditions are observed throughout the site. Where groundwater infiltration minor occurs within open excavations during construction, conventional sump pumping techniques are expected to be suitable for groundwater control.

Sediment controls should be incorporated to construction dewatering discharge outlets. These may take the form of silt bags, constructed sediment traps, or other methods of filtering the discharge water. The contractor will be responsible for regular maintenance, including sediment removal, as required. Under no circumstances will dewatering effluent be discharged directly to the receiving watercourse, without incorporating suitable measures to prevent sediment discharge, or to cause erosion or scouring at the watercourse. Both the Contractor and the Contract Administrator will be responsible for monitoring the water quality leaving the site.

As a preliminary guide, water being discharged at the site should have a turbidity level within 8 NTU of the background levels within the watercourse/municipal drain.

For construction dewatering efforts which involve a large daily volume of pumping, additional sediment control or filtration measures, such as settlement tanks may be implemented as part of the construction dewatering plan. The location of the discharge and associated sediment controls will depend on scope of work and the location of the excavation to be dewatered. Thus, the dewatering plan should be reviewed by the Owner's Engineer in consultation with the Contractor and the Contract Administrator

For projects requiring positive groundwater control with a removal rate in the range of 50,000 to 400,000 litres per day, a submission to the Environmental Activity and Sector Registry (EASR) will be required. For construction dewatering activities with volumes in excess of 400,000 litres per day, a Category 3 Permit to Take Water (PTTW) would be required for groundwater control. EASR submissions and PTTW applications are submitted to and approved by MECP according to Sections 34 and 98 of the Ontario Water Resources Act R.S.O. 1990 and Water Taking and Transfer Regulation O. Reg. 387/04.

The supporting documents to support an EASR submission or PTTW application should include calculations for the zone of influence, and identify potential qualitative and quantitative impacts to the shallow groundwater table. Details regarding volume monitoring, water quality analyses and method / location of discharge water will also be required for either type of submission. Impacts on local natural features will need to be assessed in consideration of the proposed construction dewatering plan.

Conclusion and Recommendations

The analysis completed by LDS yields the following conclusions:

- Post development peak flow rates are less than pre-development conditions;
- All storm events are contained on site;
- Sediment transport from the site during construction will be minimized; and
- An infiltration Gallery, oversized/perforated pipe system and surface storage will provide quantity control for the subject site.

We trust this letter report to be complete and meet with your acceptance. Should you have any questions concerning the findings presented herein, please do not hesitate to contact the undersigned.

Sincerely,

LDS CONSULTANTS INC.

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Principal, Community Development
o: (226) 289-2952
c: (519) 494-7785
e: anthony.gubbels@LDSConsultants.ca

encl.

- Figure 1 – Site Location Plan
- Figure 2 – Pre Development Drainage Area Plan
- Figure 3 – Post Development Drainage Area Plan
- Pre-Development Runoff Model Output
- Proposed Runoff Model Output

Z:\1614-00203 - ELMHURST, KILWORTH\DETAIL DESIGN\REPORTS\SWM\161400203 LOCATION PLAN.DWG
2019-12-13 1:40:05 PM by: TEAGAN.PRESTON



6,10,14 ELMHURST STREET
SWEID HOLDINGS INC.

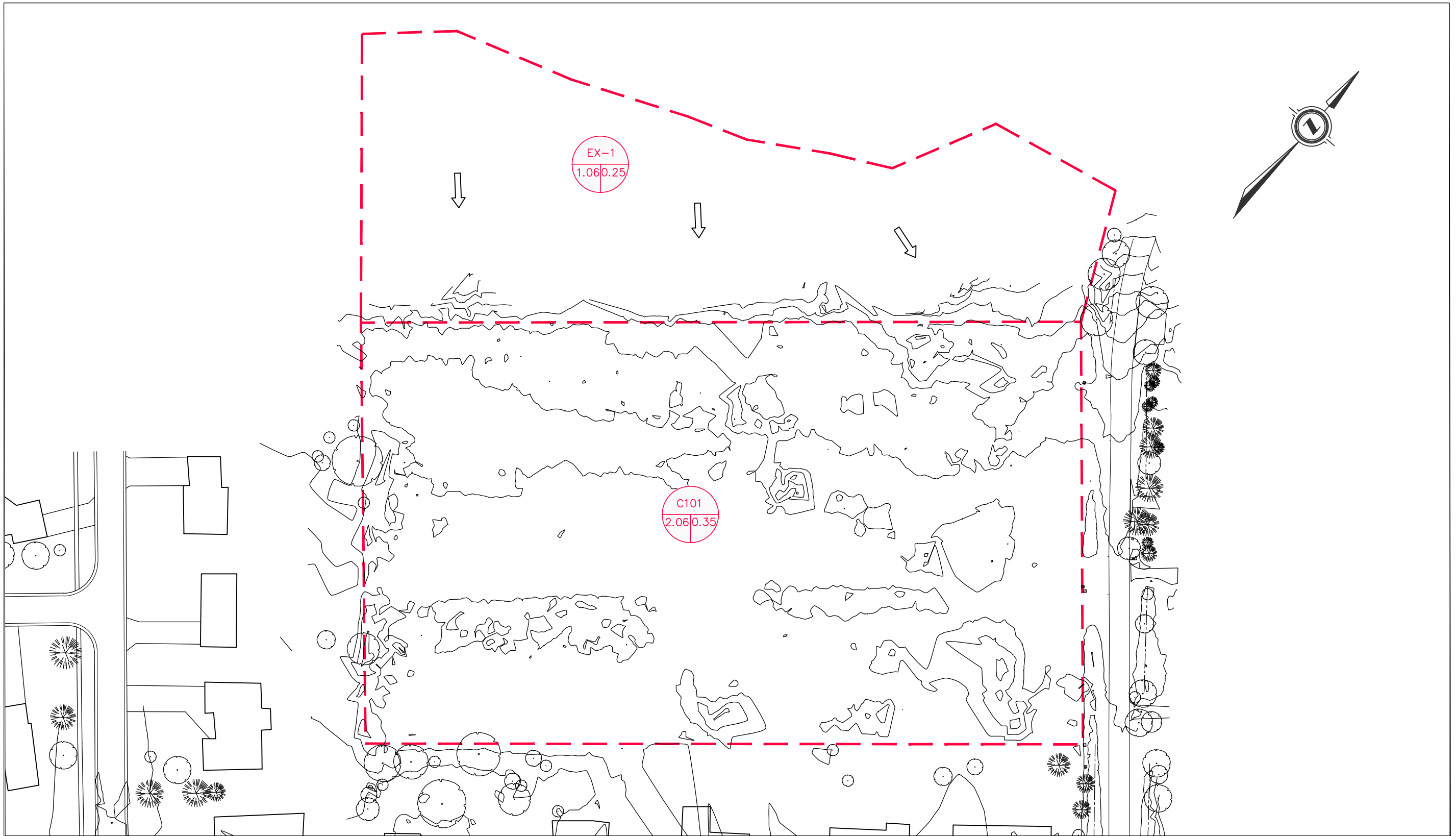
LOCATION PLAN

PROJECT: 1614-00203

SCALE: N.T.S.



FIGURE 1

H:\SWM\LD-00203\REPORTS\SWM\2020118 - LUKE SWM\ACAD\20210118 LD-00203 SWM FIGURES.DWG
1/29/2021 11:23:00 AM by: LUKE JESSON



101 CATCHMENT ID
7.6 82 AREA (ha) SCS CURVE NUMBER

LEGEND:

-  CATCHMENT AREA
-  EXISTING OVERLAND FLOW ROUTE

ELMHURST STREET
SWEID HOLDINGS INC.

PRE-DEVELOPMENT DRAINAGE AREA PLAN

PROJECT: LD-00203 SCALE: 1:1000 FIGURE 2

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00001> =====
00002>
00003> SSSSS W W M M H H Y Y M M O O 999 999 =====
00004> S W W M M M H H Y Y M M M O O 9 9 9 9
00005> SSSSS W W M M M H H H H Y Y M M M O O ## 9 9 9 9 Ver 4.05
00006> S W W M M H H Y Y M M O O 9999 9999 Sept 2011
00007> SSSSS W W M M H H Y Y M M O O 9 9 9 9 =====
00008> 9 9 9 9 # 4058874
00009> StormWater Management Hydrologic Model 999 999 =====
00010>
00011> *****
00012> ***** SWMMHYMO Ver/4.05 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHYMO-83 and OTTHYMO-89. *****
00016> *****
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Ottawa, Ontario: (613) 836-3884 *****
00019> ***** Gatineau, Quebec: (819) 243-6858 *****
00020> ***** E-Mail: swmhyo@jfsa.Com *****
00021> *****
00022>
00023> *****
00024> ***** Licensed user: Land Development Solutions *****
00025> ***** London SERIAL#:4058874 *****
00026> *****
00027> *****
00028> *****
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00030> ***** Maximum value for ID numbers : 10 *****
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00033> *****
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00042> * Summary filename: C:\LUKE_J-1\00203\LD203--1.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047> *****
00048>
00049>
00050> 001:0001-----
00051> #*****
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00053> # Date : 20-01-2021
00054> # Modeller : [LJ]
00055> # Company : LDS Consultants Inc.
00056> # License # : 4058874
00057> #*****
00058>
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00062> | METOUT= 2 (output = METRIC)
00063> | NRUN = 001
00064> | NSTORM= 0
00065>
00066> 001:0002-----
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00069> | Ptotal= 42.74 mm | B= 8.500
00070> | | C= .860
00071> | used in: INTENSITY = A / (t + B)^C
00072> |
00073> | Duration of storm = 3.00 hrs
00074> | Storm time step = 5.00 min
00075> | Time to peak ratio = .33
00076>
00077> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
00078> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
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00080> .17 3.133 .92 50.587 1.67 8.220 2.42 3.621
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00087> .75 12.574 1.50 11.323 2.25 4.136 3.00 2.531
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00089>
00090> 001:0003-----
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00093> | 01:c101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00094> | U.H. Tp(hrs)= .220
00095>
00096> Unit Hyd Qpeak (cms)= .354
00097>
00098> PEAK FLOW (cms)= .057 (i)
00099> TIME TO PEAK (hrs)= 1.333
00100> RUNOFF VOLUME (mm)= 9.041
00101> TOTAL RAINFALL (mm)= 42.745
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00103>
00104> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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00106>
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00116> | Time to peak ratio = .33
00117>
00118> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
00119> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
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00121> .17 3.511 .92 50.660 1.67 8.713 2.42 4.024
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00123> .33 4.411 1.08 65.174 1.83 6.902 2.58 3.605
00124> .42 5.072 1.17 36.212 1.92 6.254 2.67 3.428
00125> .50 5.978 1.25 24.339 2.00 5.719 2.75 3.268
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00127> .67 9.357 1.42 14.311 2.17 4.889 2.92 2.992

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00130>
00131> 001:0005-----
00132>
00133> | CALIB NASHYD | Area (ha)= 2.04 Curve Number (CN)=72.00
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00135> | U.H. Tp(hrs)= .220
00136>
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00138>
00139> PEAK FLOW (cms)= .060 (i)
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00144>
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00146>
00147>
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00158>
00159> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
00160> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
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00162> .17 3.869 .92 61.712 1.67 10.197 2.42 4.475
00163> .25 4.337 1.00 162.470 1.75 8.945 2.50 4.212
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00165> .42 5.731 1.17 44.466 1.92 7.166 2.67 3.770
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00167> .58 8.439 1.33 21.949 2.08 5.970 2.83 3.414
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00175> | 01:c101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
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00178> Unit Hyd Qpeak (cms)= .354
00179>
00180> PEAK FLOW (cms)= .088 (i)
00181> TIME TO PEAK (hrs)= 1.317
00182> RUNOFF VOLUME (mm)= 13.582
00183> TOTAL RAINFALL (mm)= 52.043
00184> RUNOFF COEFFICIENT = .261
00185>
00186> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00187>
00188>
00189> 001:0008-----
00190>
00191> | CHICAGO STORM | IDF curve parameters: A=2019.372
00192> | Ptotal= 61.48 mm | B= 9.824
00193> | | C= .875
00194> | used in: INTENSITY = A / (t + B)^C
00195> |
00196> | Duration of storm = 3.00 hrs
00197> | Storm time step = 5.00 min
00198> | Time to peak ratio = .33
00199>
00200> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
00201> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
00202> .08 3.900 .83 31.054 1.58 13.926 2.33 5.406
00203> .17 4.339 .92 74.372 1.67 11.933 2.42 5.053
00204> .25 4.890 1.00 190.819 1.75 10.414 2.50 4.743
00205> .33 5.600 1.08 95.075 1.83 9.222 2.58 4.468
00206> .42 6.545 1.17 53.763 1.92 8.265 2.67 4.224
00207> .50 7.861 1.25 35.883 2.00 7.482 2.75 4.005
00208> .58 9.802 1.33 26.308 2.08 6.831 2.83 3.808
00209> .67 12.908 1.42 20.486 2.17 6.281 2.92 3.629
00210> .75 18.531 1.50 16.634 2.25 5.811 3.00 3.466
00211>
00212>
00213> 001:0009-----
00214>
00215> | CALIB NASHYD | Area (ha)= 2.04 Curve Number (CN)=72.00
00216> | 01:c101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00217> | U.H. Tp(hrs)= .220
00218>
00219> Unit Hyd Qpeak (cms)= .354
00220>
00221> PEAK FLOW (cms)= .126 (i)
00222> TIME TO PEAK (hrs)= 1.317
00223> RUNOFF VOLUME (mm)= 18.783
00224> TOTAL RAINFALL (mm)= 61.478
00225> RUNOFF COEFFICIENT = .306
00226>
00227> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00228>
00229>
00230> 001:0010-----
00231>
00232> | CHICAGO STORM | IDF curve parameters: A=2270.665
00233> | Ptotal= 68.00 mm | B= 9.984
00234> | | C= .878
00235> | used in: INTENSITY = A / (t + B)^C
00236> |
00237> | Duration of storm = 3.00 hrs
00238> | Storm time step = 5.00 min
00239> | Time to peak ratio = .33
00240>
00241> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
00242> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
00243> .08 4.264 .83 34.467 1.58 15.391 2.33 5.928
00244> .17 4.749 .92 82.576 1.67 13.174 2.42 5.537
00245> .25 5.357 1.00 210.839 1.75 11.484 2.50 5.195
00246> .33 6.142 1.08 105.524 1.83 10.160 2.58 4.891
00247> .42 7.190 1.17 59.734 1.92 9.098 2.67 4.621
00248> .50 8.649 1.25 39.846 2.00 8.229 2.75 4.380
00249> .58 10.804 1.33 29.182 2.08 7.506 2.83 4.162
00250> .67 14.259 1.42 22.692 2.17 6.897 2.92 3.965
00251> .75 20.519 1.50 18.406 2.25 6.377 3.00 3.786
00252>
00253>
00254> 001:0011-----

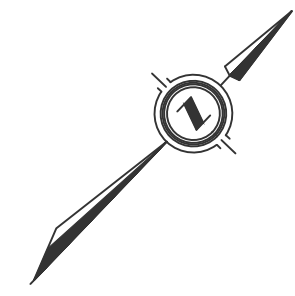
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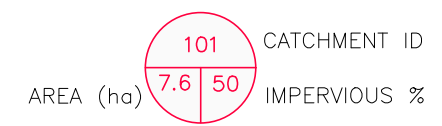
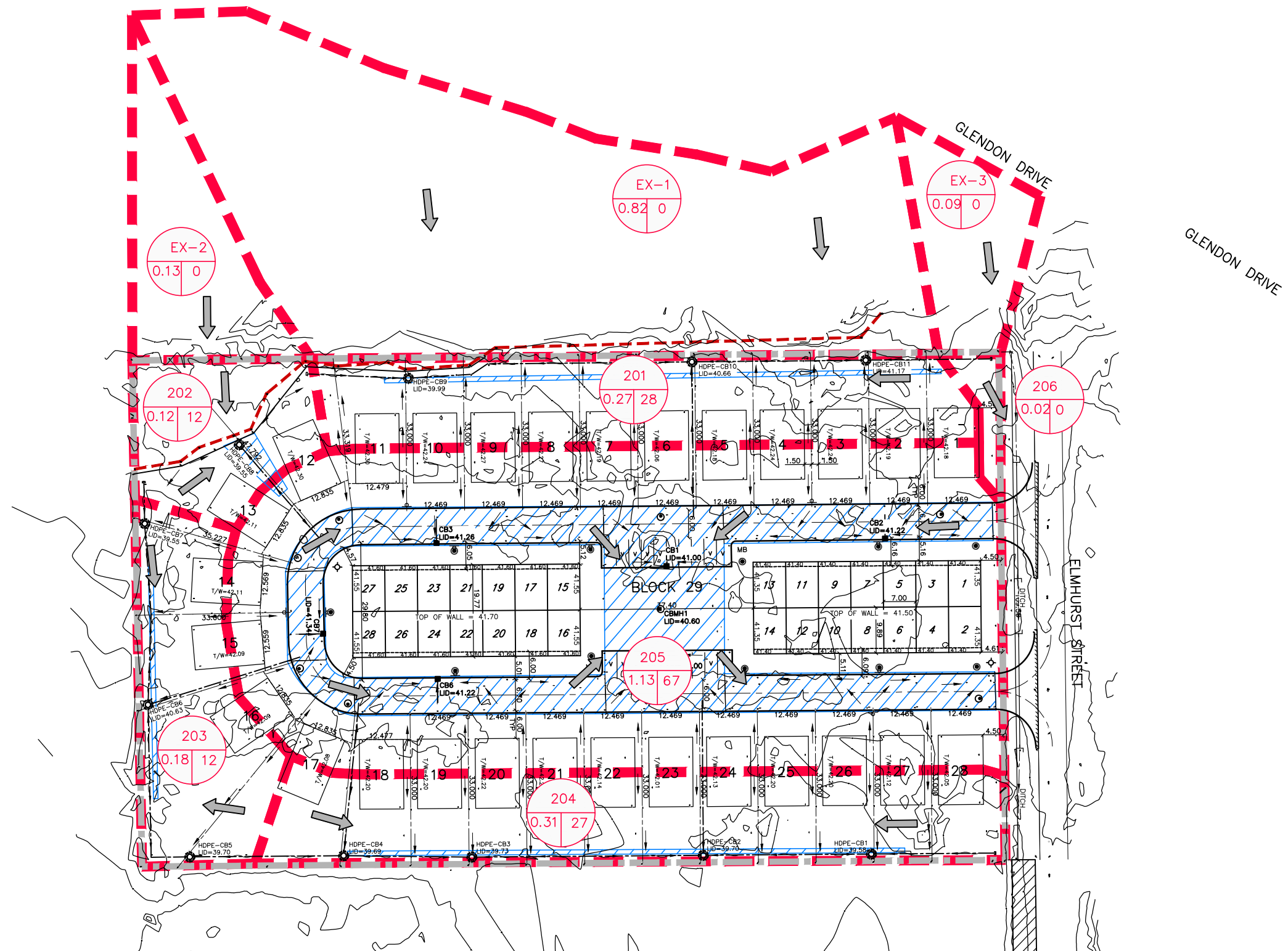
00255>-----
00256> | CALIB NASHYD | Area (ha)= 2.04 Curve Number (CN)=72.00
00257> | 01:c101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00258>-----
00259> U.H. Tp(hrs)= .220
00260> Unit Hyd Qpeak (cms)= .354
00261>
00262> PEAK FLOW (cms)= .154 (i)
00263> TIME TO PEAK (hrs)= 1.300
00264> RUNOFF VOLUME (mm)= 22.672
00265> TOTAL RAINFALL (mm)= 67.989
00266> RUNOFF COEFFICIENT = .333
00267>
00268> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00269>-----
00270>
00271> 001:0012-----
00272>
00273> | CHICAGO STORM | IDF curve parameters: A=2619.363
00274> | Ptotal= 75.83 mm | B= 10.500
00275>-----
00276> C= .884
00277> used in: INTENSITY = A / (t + B)^C
00278>
00279> Duration of storm = 3.00 hrs
00280> Storm time step = 5.00 min
00281> Time to peak ratio = .33
00282>
00283> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00284> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00285> .08 4.669 | .83 38.909 | 1.58 17.257 | 2.33 6.536
00286> .17 5.212 | .92 92.768 | 1.67 14.738 | 2.42 6.097
00287> .25 5.895 | 1.00 232.243 | 1.75 12.820 | 2.50 5.712
00288> .33 6.777 | 1.08 118.260 | 1.83 11.318 | 2.58 5.372
00289> .42 7.958 | 1.17 67.393 | 1.92 10.115 | 2.67 5.069
00290> .50 9.607 | 1.25 45.001 | 2.00 9.132 | 2.75 4.799
00291> .58 12.049 | 1.33 32.924 | 2.08 8.315 | 2.83 4.555
00292> .67 15.971 | 1.42 25.560 | 2.17 7.627 | 2.92 4.335
00293> .75 23.085 | 1.50 20.683 | 2.25 7.041 | 3.00 4.135
00294>-----
00295> 001:0013-----
00296>
00297> | CALIB NASHYD | Area (ha)= 2.04 Curve Number (CN)=72.00
00298> | 01:c101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00299>-----
00300> U.H. Tp(hrs)= .220
00301> Unit Hyd Qpeak (cms)= .354
00302>
00303> PEAK FLOW (cms)= .190 (i)
00304> TIME TO PEAK (hrs)= 1.300
00305> RUNOFF VOLUME (mm)= 27.614
00306> TOTAL RAINFALL (mm)= 75.828
00307> RUNOFF COEFFICIENT = .364
00308>
00309> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00310>-----
00311>
00312> 001:0014-----
00313>
00314> | CHICAGO STORM | IDF curve parameters: A=3048.220
00315> | Ptotal= 86.60 mm | B= 10.030
00316>-----
00317> C= .888
00318> used in: INTENSITY = A / (t + B)^C
00319>
00320> Duration of storm = 3.00 hrs
00321> Storm time step = 5.00 min
00322> Time to peak ratio = .33
00323>
00324> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00325> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00326> .17 5.748 | .92 106.492 | 1.67 16.378 | 2.42 6.730
00327> .25 6.505 | 1.00 274.730 | 1.75 14.228 | 2.50 6.302
00328> .33 7.486 | 1.08 136.493 | 1.83 12.548 | 2.58 5.925
00329> .42 8.799 | 1.17 76.628 | 1.92 11.204 | 2.67 5.589
00330> .50 10.637 | 1.25 50.754 | 2.00 10.107 | 2.75 5.289
00331> .58 13.366 | 1.33 36.943 | 2.08 9.197 | 2.83 5.020
00332> .67 17.763 | 1.42 28.579 | 2.17 8.431 | 2.92 4.776
00333> .75 25.782 | 1.50 23.067 | 2.25 7.779 | 3.00 4.554
00334>-----
00335>
00336> 001:0015-----
00337>
00338> | CALIB NASHYD | Area (ha)= 2.04 Curve Number (CN)=72.00
00339> | 01:c101 DT= 1.00 | Ia (mm)= 8.000 # of Linear Res.(N)= 3.00
00340>-----
00341> U.H. Tp(hrs)= .220
00342> Unit Hyd Qpeak (cms)= .354
00343>
00344> PEAK FLOW (cms)= .247 (i)
00345> TIME TO PEAK (hrs)= 1.300
00346> RUNOFF VOLUME (mm)= 34.829
00347> TOTAL RAINFALL (mm)= 86.599
00348> RUNOFF COEFFICIENT = .402
00349>
00350> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00351>-----
00352>
00353> 001:0016-----
00354> FINISH
00355>-----
00356> *****
00357> WARNINGS / ERRORS / NOTES
00358>-----
00359> Simulation ended on 2021-01-20 at 15:16:31
00360>-----
00361>
00362>

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Z:\1614-00203 - ELMHURST, KILWORTH\DETAIL DESIGN\REPORTS\SWM\20210707 SITE PLAN REVISION\20210707 LD-00203 SWM FIGURES.DWG
 7/8/2021 8:12:54 AM by: JOE.VANDENBERG



KILWORTH PARK DRIVE



- LEGEND:
- CATCHMENT AREA
 - PROPOSED OVERLAND FLOW ROUTE
 - PROPOSED INFILTRATION MEASURES

MN 6, 10, 14 ELMHURST STREET
 SWEID HOLDINGS INC.

POST DEVELOPMENT DRAINAGE AREA PLAN

PROJECT: 1614-00203 SCALE: N.T.S. **FIGURE 2**

```

00001> =====
00002>
00003> SSSSS W W M M H H Y Y M M M OOO 999 999 =====
00004> S W W M M M H H Y Y M M M O O 9 9 9 9 9
00005> SSSSS W W M M M H H H H Y Y M M M O O ## 9 9 9 9 Ver 4.05
00006> S W W M M H H H Y Y M M O O 9999 9999 Sept 2011
00007> SSSSS W W M M H H Y Y M M OOO 9 9 9 =====
00008> # 4058874
00009> StormWater Management Hydrologic Model 999 999 =====
00010>
00011> *****
00012> ***** SWMMHYMO Ver/4.05 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHYMO-83 and OTTHYMO-89. *****
00016> *****
00017> ***** Distributed by: J. F. Sabourin and Associates Inc. *****
00018> ***** Ottawa, Ontario: (613) 836-3884 *****
00019> ***** Gatineau, Quebec: (819) 243-6858 *****
00020> ***** E-Mail: swmhymo@jfsa.Com *****
00021> *****
00022>
00023> ***** Licensed user: Land Development Solutions *****
00024> ***** London SERIAL#:4058874 *****
00025> *****
00026> *****
00027> *****
00028> *****
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 105408 *****
00032> ***** Max. number of flow points : 105408 *****
00033> *****
00034> *****
00035> *****
00036> ***** D E T A I L E D O U T P U T *****
00037> *****
00038> * DATE: 2021-07-14 TIME: 12:15:13 RUN COUNTER: 000595 *
00039> *****
00040> * Input filename: C:\SWMHYMO\PROJECTS\00203\210707PC.dat *
00041> * Output filename: C:\SWMHYMO\PROJECTS\00203\210707PC.out *
00042> * Summary filename: C:\SWMHYMO\PROJECTS\00203\210707PC.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047> *****
00048>
00049>
00050> 001:0001-----
00051> # Project Name: [Elmhurst] Project Number: [LD-00203]
00052> # Date : 26-11-2019
00053> # Modeller : [JV]
00054> # Company : LDS Consultants Inc.
00055> # License # : 4058874
00056> # *****
00057> # *****
00058> # *****
00059> | START | Project dir.: C:\SWMHYMO\PROJECTS\00203\
00060> | Rainfall dir.: C:\SWMHYMO\PROJECTS\00203\
00061> | TZERO = .00 hrs on 0
00062> | METOUT= 2 (output = METRIC)
00063> | NRUN = 001
00064> | NSTORM= 0
00065> | *****
00066> 001:0002-----
00067> | CHICAGO STORM | IDF curve parameters: A= 538.850
00068> | Ptotal= 23.54 mm | B= 6.331
00069> | C= .809
00070> | used in: INTENSITY = A / (t + B)^C
00071> |
00072> | Duration of storm = 3.00 hrs
00073> | Storm time step = 5.00 min
00074> | Time to peak ratio = .33
00075> | *****
00076> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00077> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00078> | .08 1.881 | .83 10.955 | 1.58 5.429 | 2.33 2.467
00079> | .17 2.056 | .92 25.749 | 1.67 4.765 | 2.42 2.332
00080> | .25 2.270 | 1.00 75.607 | 1.75 4.251 | 2.50 2.213
00081> | .33 2.539 | 1.08 33.150 | 1.83 3.841 | 2.58 2.106
00082> | .42 2.889 | 1.17 18.381 | 1.92 3.506 | 2.67 2.010
00083> | .50 3.363 | 1.25 12.506 | 2.00 3.228 | 2.75 1.923
00084> | .58 4.040 | 1.33 9.432 | 2.08 2.994 | 2.83 1.844
00085> | .67 5.090 | 1.42 7.564 | 2.17 2.793 | 2.92 1.772
00086> | .75 6.931 | 1.50 6.317 | 2.25 2.619 | 3.00 1.705
00087> | *****
00088> | *****
00089> | *****
00090> 001:0003-----
00091> | CHICAGO STORM | IDF curve parameters: A=1290.000
00092> | Ptotal= 42.74 mm | B= 8.500
00093> | C= .860
00094> | used in: INTENSITY = A / (t + B)^C
00095> |
00096> | Duration of storm = 3.00 hrs
00097> | Storm time step = 5.00 min
00098> | Time to peak ratio = .33
00099> | *****
00100> | *****
00101> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00102> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00103> | .08 2.831 | .83 20.863 | 1.58 9.536 | 2.33 3.861
00104> | .17 3.133 | .92 50.587 | 1.67 8.220 | 2.42 3.621
00105> | .25 3.510 | 1.00 137.563 | 1.75 7.213 | 2.50 3.410
00106> | .33 3.992 | 1.08 65.095 | 1.83 6.422 | 2.58 3.222
00107> | .42 4.631 | 1.17 36.145 | 1.92 5.784 | 2.67 3.054
00108> | .50 5.513 | 1.25 24.074 | 2.00 5.260 | 2.75 2.904
00109> | .58 6.807 | 1.33 17.708 | 2.08 4.823 | 2.83 2.767
00110> | .67 8.864 | 1.42 13.862 | 2.17 4.453 | 2.92 2.644
00111> | .75 12.574 | 1.50 11.323 | 2.25 4.136 | 3.00 2.531
00112> | *****
00113> | *****
00114> 001:0004-----
00115> | *****
00116> | CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
00117> | 01:EX1 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00118> | U.H. Tp(hrs)= .500
00119> | *****
00120> | Unit Hyd Qpeak (cms)= .079
00121> | *****
00122> | PEAK FLOW (cms)= .005 (i)
00123> | TIME TO PEAK (hrs)= 1.667
00124> | RUNOFF VOLUME (mm)= 2.466
00125> | TOTAL RAINFALL (mm)= 42.745
00126> | RUNOFF COEFFICIENT = .058
00127> | *****

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00128> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00129>
00130> -----
00131> 001:0005-----
00132>
00133> CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
00134> 02:EX2 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00135> | U.H. Tp(hrs)= .500
00136>
00137> | Unit Hyd Qpeak (cms)= .079
00138> | *****
00139> | PEAK FLOW (cms)= .005 (i)
00140> | TIME TO PEAK (hrs)= 1.667
00141> | RUNOFF VOLUME (mm)= 2.466
00142> | TOTAL RAINFALL (mm)= 42.745
00143> | RUNOFF COEFFICIENT = .058
00144> | *****
00145> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00146>
00147> -----
00148> 001:0006-----
00149>
00150> CALIB STANDHYD | Area (ha)= .27
00151> 03:C201 DT= 1.00 | Total Imp(%)= 28.00 Dir. Conn.(%)= 5.00
00152> | *****
00153> | IMPERVIOUS PERVIOUS (i)
00154> | Surface Area (ha)= .08 .19
00155> | Dep. Storage (mm)= 2.00 5.00
00156> | Average Slope (%)= 1.80 1.80
00157> | Length (m)= 40.00 10.00
00158> | Mannings n = .013 .025
00159> | *****
00160> | Max.eff.Inten.(mm/hr)= 137.56 31.31
00161> | over (min) 1.00 2.00
00162> | Storage Coeff. (min)= 1.09 (ii) 2.36 (ii)
00163> | Unit Hyd. Tpeak (min)= 1.00 2.00
00164> | Unit Hyd. peak (cms)= 1.02 .50
00165> | *****
00166> | PEAK FLOW (cms)= .01 .01 *TOTALS*
00167> | TIME TO PEAK (hrs)= 1.00 1.02 .018 (iii)
00168> | RUNOFF VOLUME (mm)= 40.74 7.72 9.375
00169> | TOTAL RAINFALL (mm)= 42.74 42.74 42.745
00170> | RUNOFF COEFFICIENT = .95 .18 .219
00171> | *****
00172> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00173> CN* = 55.0 Ia = Dep. Storage (Above)
00174> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00175> THAN THE STORAGE COEFFICIENT.
00176> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00177>
00178> -----
00179> 001:0007-----
00180>
00181> | ADD HYD (201trench ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00182> | (ha) (cms) (hrs) (mm) (cms)
00183> | ID1 01:EX1 1.04 .005 1.67 2.47 .000
00184> | +ID2 03:C201 .27 .018 1.00 9.38 .000
00185> | *****
00186> | SUM 10:201trench 1.31 .018 1.00 3.89 .000
00187> | *****
00188> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00189> | *****
00190> | *****
00191> 001:0008-----
00192>
00193> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00194> TotalHyd 10:201tre | Number of inlets in system [NINLET] = 1
00195> | Total minor system capacity = .001 (cms)
00196> | Total major system storage [TMJSTO] = 108. (cu.m.)
00197> | *****
00198> | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00199> | (ha) (cms) (hrs) (mm) (cms)
00200> | TOTAL HYD. 10:201tre 1.31 .018 1.000 3.890 .000
00201> | *****
00202> | MAJOR SYST 01:201OVL .00 .000 .000 .000 .000
00203> | MINOR SYST 03:INFIL 1.31 .001 .800 3.892 .000
00204> | *****
00205> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00206> | *****
00207> | Maximum MAJOR SYSTEM storage used = 41. (cu.m.)
00208> | *****
00209> | *****
00210> 001:0009-----
00211>
00212> CALIB NASHYD | Area (ha)= .12 Curve Number (CN)=55.00
00213> 04:202 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00214> | U.H. Tp(hrs)= .500
00215> | *****
00216> | Unit Hyd Qpeak (cms)= .009
00217> | *****
00218> | PEAK FLOW (cms)= .001 (i)
00219> | TIME TO PEAK (hrs)= 1.667
00220> | RUNOFF VOLUME (mm)= 5.793
00221> | TOTAL RAINFALL (mm)= 42.745
00222> | RUNOFF COEFFICIENT = .136
00223> | *****
00224> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00225>
00226> -----
00227> 001:0010-----
00228>
00229> | ADD HYD (202trench ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00230> | (ha) (cms) (hrs) (mm) (cms)
00231> | ID1 02:EX2 1.04 .005 1.67 2.47 .000
00232> | +ID2 04:202 .12 .001 1.67 5.79 .000
00233> | *****
00234> | SUM 09:202trench 1.16 .006 1.67 2.81 .000
00235> | *****
00236> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00237> | *****
00238> | *****
00239> 001:0011-----
00240>
00241> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
00242> TotalHyd 09:202tre | Number of inlets in system [NINLET] = 1
00243> | Total minor system capacity = .000 (cms)
00244> | Total major system storage [TMJSTO] = 7. (cu.m.)
00245> | *****
00246> | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00247> | (ha) (cms) (hrs) (mm) (cms)
00248> | TOTAL HYD. 09:202tre 1.16 .006 1.667 2.810 .000
00249> | *****
00250> | MAJOR SYST 02:202OVL .76 .006 1.667 2.810 .000
00251> | MINOR SYST 04:INFIL .40 .000 1.000 2.822 .000
00252> | *****
00253> | NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00254> | *****

```

00255> Maximum MAJOR SYSTEM storage used = 7. (cu.m.)

00256> -----

00257> 001:0012

00258> CALIB NASHYD Area (ha)= .18 Curve Number (CN)=55.00

00261> 05:203 DT= 5.00 Ia (mm)= 5.000 # of Linear Res.(N)= 3.00

00262> U.H. Tp(hrs)= .500

00263> Unit Hyd Qpeak (cms)= .014

00264> PEAK FLOW (cms)= .002 (i)

00267> TIME TO PEAK (hrs)= 1.667

00268> RUNOFF VOLUME (mm)= 5.797

00269> TOTAL RAINFALL (mm)= 42.745

00270> RUNOFF COEFFICIENT = .136

00271> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00272> -----

00273> 001:0013

00276> COMPUTE DUALHYD Average inlet capacities [CINLET] = .000 (cms)

00277> TotalHyd 05:203 Number of inlets in system [NINLET] = 1

00279> Total minor system capacity = .000 (cms)

00280> Total major system storage [TMJSTO] = 40. (cu.m.)

00281> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00282> (ha) (cms) (hrs) (mm) (cms)

00284> TOTAL HYD. 05:203 .18 .002 1.667 5.797 .000

00285> MAJOR SYST 03:203OVL .00 .000 .000 .000 .000

00286> MINOR SYST 04:INFIL .18 .000 1.083 5.797 .000

00287> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00289> Maximum MAJOR SYSTEM storage used = 7. (cu.m.)

00290> -----

00291> 001:0014

00296> CALIB STANDHYD Area (ha)= .31

00297> 06:C204 DT= 1.00 Total Imp(%)= 31.00 Dir. Conn.(%)= 5.00

00298> IMPERVIOUS PERVIOUS (i)

00300> Surface Area (ha)= .10 .21

00301> Dep. Storage (mm)= 2.00 5.00

00302> Average Slope (%)= 1.80 1.80

00303> Length (m)= 40.00 10.00

00304> Mannings n = .013 .025

00305> Max. eff. Inten. (mm/hr)= 137.56 34.19

00307> over (min) 1.00 2.00

00308> Storage Coeff. (min)= 1.09 (ii) 2.31 (ii)

00309> Unit Hyd. Tpeak (min)= 1.00 2.00

00310> Unit Hyd. peak (cms)= 1.02 .51

00311> *TOTALS*

00312> PEAK FLOW (cms)= .01 .02 .021 (iii)

00313> TIME TO PEAK (hrs)= 1.00 1.02 1.000

00314> RUNOFF VOLUME (mm)= 40.74 8.05 9.684

00315> TOTAL RAINFALL (mm)= 42.74 42.74 42.745

00316> RUNOFF COEFFICIENT = .95 .19 .227

00317> (i) CN PROCEDURE SELECTED FOR PEROUS LOSSES:

00318> CN* = 55.0 Ia = Dep. Storage (Above)

00320> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

00321> THAN THE STORAGE COEFFICIENT.

00322> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00323> -----

00324> 001:0015

00326> COMPUTE DUALHYD Average inlet capacities [CINLET] = .001 (cms)

00328> TotalHyd 06:C204 Number of inlets in system [NINLET] = 1

00329> Total minor system capacity = .001 (cms)

00330> Total major system storage [TMJSTO] = 108. (cu.m.)

00331> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00332> (ha) (cms) (hrs) (mm) (cms)

00334> TOTAL HYD. 06:C204 .31 .021 1.000 9.684 .000

00335> MAJOR SYST 05:204OVL .00 .000 .000 .000 .000

00336> MINOR SYST 04:INFIL .31 .001 .783 9.684 .000

00337> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00338> Maximum MAJOR SYSTEM storage used = 23. (cu.m.)

00339> -----

00340> 001:0016

00346> CALIB STANDHYD Area (ha)= 1.13

00347> 07:C205 DT= 1.00 Total Imp(%)= 67.00 Dir. Conn.(%)= 35.00

00348> IMPERVIOUS PERVIOUS (i)

00349> Surface Area (ha)= .76 .37

00351> Dep. Storage (mm)= 2.00 5.00

00352> Average Slope (%)= 1.80 1.80

00353> Length (m)= 40.00 10.00

00354> Mannings n = .013 .025

00355> Max. eff. Inten. (mm/hr)= 137.56 69.61

00356> over (min) 1.00 2.00

00357> Storage Coeff. (min)= 1.09 (ii) 2.01 (ii)

00359> Unit Hyd. Tpeak (min)= 1.00 2.00

00360> Unit Hyd. peak (cms)= 1.02 .56

00361> *TOTALS*

00362> PEAK FLOW (cms)= .15 .06 .207 (iii)

00363> TIME TO PEAK (hrs)= 1.00 1.02 1.000

00364> RUNOFF VOLUME (mm)= 40.74 11.09 21.472

00365> TOTAL RAINFALL (mm)= 42.74 42.74 42.745

00366> RUNOFF COEFFICIENT = .95 .26 .502

00367> (i) CN PROCEDURE SELECTED FOR PEROUS LOSSES:

00368> CN* = 55.0 Ia = Dep. Storage (Above)

00369> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

00370> THAN THE STORAGE COEFFICIENT.

00371> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00372> -----

00373> 001:0017

00376> ADD HYD (to205) ID: NHYD AREA QPEAK TPEAK R.V. DWF

00377> (ha) (cms) (hrs) (mm) (cms)

00379> ID1 01:201OVL .00 .000 .00 .00 .000

00380> +ID2 02:202OVL .76 .006 1.67 2.81 .000

00381> +ID3 03:203OVL .00 .000 .00 .00 .000

00382> +ID4 05:204OVL .00 .000 .00 .00 .000

00383> +ID5 07:C205 1.13 .207 1.00 21.47 .000

00384> SUM 10:to205 1.89 .207 1.00 13.96 .000

00385> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00386> -----

00387> 001:0018

00392> COMPUTE DUALHYD Average inlet capacities [CINLET] = .023 (cms)

00393> TotalHyd 10:to205 Number of inlets in system [NINLET] = 1

00394> Total minor system capacity = .023 (cms)

00395> Total major system storage [TMJSTO] = 761. (cu.m.)

00396> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00397> (ha) (cms) (hrs) (mm) (cms)

00399> TOTAL HYD. 10:to205 1.89 .207 1.000 13.959 .000

00400> MAJOR SYST 01:205OVL .00 .000 .000 .000 .000

00401> MINOR SYST 02:INFIL 1.89 .023 .800 14.019 .000

00402> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00403> Maximum MAJOR SYSTEM storage used = 118. (cu.m.)

00404> -----

00405> 001:0019

00411> CALIB NASHYD Area (ha)= .09 Curve Number (CN)=32.00

00412> 07:EX3 DT= 5.00 Ia (mm)= 5.000 # of Linear Res.(N)= 3.00

00413> U.H. Tp(hrs)= .500

00414> Unit Hyd Qpeak (cms)= .007

00415> PEAK FLOW (cms)= .000 (i)

00417> TIME TO PEAK (hrs)= 1.667

00418> RUNOFF VOLUME (mm)= 2.455

00419> TOTAL RAINFALL (mm)= 42.745

00420> RUNOFF COEFFICIENT = .057

00421> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00422> -----

00423> 001:0020

00427> CALIB NASHYD Area (ha)= .05 Curve Number (CN)=55.00

00428> 04:206 DT= 5.00 Ia (mm)= 5.000 # of Linear Res.(N)= 3.00

00429> U.H. Tp(hrs)= .250

00430> Unit Hyd Qpeak (cms)= .008

00431> PEAK FLOW (cms)= .001 (i)

00432> TIME TO PEAK (hrs)= 1.333

00433> RUNOFF VOLUME (mm)= 5.789

00434> TOTAL RAINFALL (mm)= 42.745

00435> RUNOFF COEFFICIENT = .135

00436> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00437> -----

00438> 001:0021

00444> ADD HYD (toDitch) ID: NHYD AREA QPEAK TPEAK R.V. DWF

00445> (ha) (cms) (hrs) (mm) (cms)

00447> ID1 04:206 .05 .001 1.33 5.79 .000

00448> +ID2 07:EX3 .09 .000 1.67 2.46 .000

00449> SUM 10:toDitch .14 .001 1.42 3.65 .000

00450> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00451> -----

00452> 001:0022

00457> CHICAGO STORM IDF curve parameters: A=1183.740

00458> Ptotal= 44.18 mm B= 7.641

00459> C= 838

00460> used in: INTENSITY = A / (T + B)^C

00461> Duration of storm = 3.00 hrs

00462> Storm time step = 5.00 min

00463> Time to peak ratio = .33

00464> TIME RAIN TIME RAIN TIME RAIN TIME RAIN

00465> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

00466> .08 3.191 .83 21.189 1.58 10.028 2.33 4.275

00467> .17 3.511 .92 50.660 1.67 8.713 2.42 4.024

00468> .25 3.908 1.00 141.242 1.75 7.702 2.50 3.802

00469> .33 4.411 1.08 65.174 1.83 6.902 2.58 3.605

00470> .42 5.072 1.17 36.212 1.92 6.254 2.67 3.428

00471> .50 5.978 1.25 24.339 2.00 5.719 2.75 3.268

00472> .58 7.291 1.33 18.094 2.08 5.271 2.83 3.124

00473> .67 9.357 1.42 14.311 2.17 4.889 2.92 2.992

00474> .75 13.038 1.50 11.802 2.25 4.561 3.00 2.871

00475> -----

00476> 001:0023

00481> CALIB NASHYD Area (ha)= 1.04 Curve Number (CN)=32.00

00482> 01:EX1 DT= 5.00 Ia (mm)= 5.000 # of Linear Res.(N)= 3.00

00483> U.H. Tp(hrs)= .500

00484> Unit Hyd Qpeak (cms)= .079

00485> PEAK FLOW (cms)= .005 (i)

00486> TIME TO PEAK (hrs)= 1.667

00487> RUNOFF VOLUME (mm)= 2.651

00488> TOTAL RAINFALL (mm)= 44.184

00489> RUNOFF COEFFICIENT = .060

00490> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00491> -----

00492> 001:0024

00497> CALIB NASHYD Area (ha)= 1.04 Curve Number (CN)=32.00

00498> 02:EX2 DT= 5.00 Ia (mm)= 5.000 # of Linear Res.(N)= 3.00

00499> U.H. Tp(hrs)= .500

00500> Unit Hyd Qpeak (cms)= .079

00501> PEAK FLOW (cms)= .005 (i)

00502> TIME TO PEAK (hrs)= 1.667

00503> RUNOFF VOLUME (mm)= 2.651

00504> TOTAL RAINFALL (mm)= 44.184

00505> RUNOFF COEFFICIENT = .060

00509> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00510> -----

00511> | CALIB STANDHYD | Area (ha)= .27

00512> | 03:C201 DT= 1.00 | Total Imp(%)= 28.00 Dir. Conn.(%)= 5.00

00513> -----

00514> IMPERVIOUS PERVIOUS (i)

00515> Surface Area (ha)= .08 .19

00516> Dep. Storage (mm)= 2.00 5.00

00517> Average Slope (%)= 1.80 1.80

00518> Length (m)= 40.00 10.00

00519> Mannings n = .013 .025

00520> Max.eff.Inten.(mm/hr)= 141.24 33.22

00521> over (min)= 1.00 2.00

00522> Storage Coeff. (min)= 1.08 (ii) 2.31 (ii)

00523> Unit Hyd. Tpeak (min)= 1.00 2.00

00524> Unit Hyd. peak (cms)= 1.03 .51

00525> -----

00526> *TOTALS*

00527> PEAK FLOW (cms)= .01 .01 .019 (iii)

00528> TIME TO PEAK (hrs)= 1.00 1.02 1.000

00529> RUNOFF VOLUME (mm)= 42.18 8.25 9.942

00530> TOTAL RAINFALL (mm)= 44.18 44.18 44.184

00531> RUNOFF COEFFICIENT = .95 .19 .225

00532> -----

00533> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

00534> CN* = 55.0 Ia = Dep. Storage (Above)

00535> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

00536> THAN THE STORAGE COEFFICIENT.

00537> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00538> -----

00539> 001:0025-----

00540> | ADD HYD (201:trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

00541> | 01:EX1 | 1.04 .005 1.67 2.65 .000

00542> | +ID2 03:C201 | .27 .019 1.00 9.94 .000

00543> -----

00544> SUM 10:201trench 1.31 .019 1.00 4.15 .000

00545> -----

00546> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00547> -----

00548> 001:0027-----

00549> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)

00550> | TotalHyd 10:201tre | Number of inlets in system [NINLET] = 1

00551> | | Total minor system capacity = .001 (cms)

00552> | | Total major system storage [TMJSTO] = 108.(cu.m.)

00553> -----

00554> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00555> (ha) (cms) (hrs) (mm) (cms)

00556> TOTAL HYD. 10:201tre 1.31 .019 1.000 4.154 .000

00557> MAJOR SYST 01:201OVL .00 .000 .000 .000 .000

00558> MINOR SYST 03:INFIL 1.31 .001 .783 4.154 .000

00559> -----

00560> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00561> -----

00562> 001:0028-----

00563> | CALIB NASHYD | Area (ha)= .12 Curve Number (CN)=55.00

00564> | 04:202 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00

00565> | U.H. Tp(hrs)= .500

00566> -----

00567> Unit Hyd Qpeak (cms)= .009

00568> -----

00569> PEAK FLOW (cms)= .001 (i)

00570> TIME TO PEAK (hrs)= 1.667

00571> RUNOFF VOLUME (mm)= 6.208

00572> TOTAL RAINFALL (mm)= 44.184

00573> RUNOFF COEFFICIENT = .141

00574> -----

00575> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00576> -----

00577> 001:0029-----

00578> | ADD HYD (202:trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

00579> | 01:EX2 | 1.04 .005 1.67 2.65 .000

00580> | +ID2 04:202 | .12 .001 1.67 6.21 .000

00581> -----

00582> SUM 09:202trench 1.16 .006 1.67 3.02 .000

00583> -----

00584> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00585> -----

00586> 001:0030-----

00587> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)

00588> | TotalHyd 09:202tre | Number of inlets in system [NINLET] = 1

00589> | | Total minor system capacity = .000 (cms)

00590> | | Total major system storage [TMJSTO] = 7.(cu.m.)

00591> -----

00592> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00593> (ha) (cms) (hrs) (mm) (cms)

00594> TOTAL HYD. 09:202tre 1.16 .006 1.667 3.019 .000

00595> MAJOR SYST 02:202OVL .79 .006 1.667 3.019 .000

00596> MINOR SYST 04:INFIL .37 .000 1.000 3.042 .000

00597> -----

00598> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00599> -----

00600> Maximum MAJOR SYSTEM storage used = 7.(cu.m.)

00601> -----

00602> 001:0031-----

00603> | CALIB NASHYD | Area (ha)= .18 Curve Number (CN)=55.00

00604> | 05:203 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00

00605> | U.H. Tp(hrs)= .500

00606> -----

00607> Unit Hyd Qpeak (cms)= .014

00608> -----

00609> PEAK FLOW (cms)= .002 (i)

00610> TIME TO PEAK (hrs)= 1.667

00611> RUNOFF VOLUME (mm)= 6.210

00612> TOTAL RAINFALL (mm)= 44.184

00613> RUNOFF COEFFICIENT = .141

00636> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00637> -----

00638> 001:0032-----

00639> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)

00640> | TotalHyd 05:203 | Number of inlets in system [NINLET] = 1

00641> | | Total minor system capacity = .000 (cms)

00642> | | Total major system storage [TMJSTO] = 40.(cu.m.)

00643> -----

00644> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00645> (ha) (cms) (hrs) (mm) (cms)

00646> TOTAL HYD. 05:203 .18 .002 1.667 6.210 .000

00647> MAJOR SYST 03:203OVL .00 .000 .000 .000 .000

00648> MINOR SYST 04:INFIL .18 .000 1.083 6.266 .000

00649> -----

00650> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00651> -----

00652> Maximum MAJOR SYSTEM storage used = 8.(cu.m.)

00653> -----

00654> 001:0033-----

00655> | CALIB STANDHYD | Area (ha)= .31

00656> | 06:C204 DT= 1.00 | Total Imp(%)= 31.00 Dir. Conn.(%)= 5.00

00657> -----

00658> IMPERVIOUS PERVIOUS (i)

00659> Surface Area (ha)= .10 .21

00660> Dep. Storage (mm)= 2.00 5.00

00661> Average Slope (%)= 1.80 1.80

00662> Length (m)= 40.00 10.00

00663> Mannings n = .013 .025

00664> Max.eff.Inten.(mm/hr)= 141.24 36.25

00665> over (min)= 1.00 2.00

00666> Storage Coeff. (min)= 1.08 (ii) 2.27 (ii)

00667> Unit Hyd. Tpeak (min)= 1.00 2.00

00668> Unit Hyd. peak (cms)= 1.03 .51

00669> -----

00670> *TOTALS*

00671> PEAK FLOW (cms)= .01 .02 .022 (iii)

00672> TIME TO PEAK (hrs)= 1.00 1.02 1.000

00673> RUNOFF VOLUME (mm)= 42.18 8.59 10.268

00674> TOTAL RAINFALL (mm)= 44.18 44.18 44.184

00675> RUNOFF COEFFICIENT = .95 .19 .232

00676> -----

00677> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

00678> CN* = 55.0 Ia = Dep. Storage (Above)

00679> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

00680> THAN THE STORAGE COEFFICIENT.

00681> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00682> -----

00683> 001:0034-----

00684> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)

00685> | TotalHyd 06:C204 | Number of inlets in system [NINLET] = 1

00686> | | Total minor system capacity = .001 (cms)

00687> | | Total major system storage [TMJSTO] = 108.(cu.m.)

00688> -----

00689> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00690> (ha) (cms) (hrs) (mm) (cms)

00691> TOTAL HYD. 06:C204 .31 .022 1.000 10.268 .000

00692> MAJOR SYST 05:204OVL .00 .000 .000 .000 .000

00693> MINOR SYST 04:INFIL .31 .001 .767 10.281 .000

00694> -----

00695> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00696> -----

00697> Maximum MAJOR SYSTEM storage used = 25.(cu.m.)

00698> -----

00699> 001:0035-----

00700> | CALIB STANDHYD | Area (ha)= 1.13

00701> | 07:C205 DT= 1.00 | Total Imp(%)= 67.00 Dir. Conn.(%)= 35.00

00702> -----

00703> IMPERVIOUS PERVIOUS (i)

00704> Surface Area (ha)= .76 .37

00705> Dep. Storage (mm)= 2.00 5.00

00706> Average Slope (%)= 1.80 1.80

00707> Length (m)= 40.00 10.00

00708> Mannings n = .013 .025

00709> Max.eff.Inten.(mm/hr)= 141.24 73.49

00710> over (min)= 1.00 2.00

00711> Storage Coeff. (min)= 1.08 (ii) 1.98 (ii)

00712> Unit Hyd. Tpeak (min)= 1.00 2.00

00713> Unit Hyd. peak (cms)= 1.03 .56

00714> -----

00715> *TOTALS*

00716> PEAK FLOW (cms)= .15 .06 .215 (iii)

00717> TIME TO PEAK (hrs)= 1.00 1.02 1.000

00718> RUNOFF VOLUME (mm)= 42.18 11.79 22.425

00719> TOTAL RAINFALL (mm)= 44.18 44.18 44.184

00720> RUNOFF COEFFICIENT = .95 .27 .508

00721> -----

00722> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

00723> CN* = 55.0 Ia = Dep. Storage (Above)

00724> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

00725> THAN THE STORAGE COEFFICIENT.

00726> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

00727> -----

00728> 001:0036-----

00729> | ADD HYD (to205) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

00730> | 01:201OVL | .00 .000 .00 .00 .000

00731> | +ID2 02:203OVL | .79 .006 1.67 3.02 .000

00732> | +ID3 03:203OVL | .00 .000 .00 .00 .000

00733> | +ID4 05:204OVL | .00 .000 .00 .00 .000

00734> | +ID5 07:C205 | 1.13 .215 1.00 22.43 .000

00735> -----

00736> SUM 10:to205 1.92 .215 1.00 14.46 .000

00737> -----

00738> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

00739> -----

00740> 001:0037-----

00741> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .023 (cms)

00742> | TotalHyd 10:to205 | Number of inlets in system [NINLET] = 1

00743> | | Total minor system capacity = .023 (cms)

00744> | | Total major system storage [TMJSTO] = 761.(cu.m.)

00745> -----

00746> ID: NHYD AREA QPEAK TPEAK R.V. DWF

00763> (ha) (cms) (hrs) (mm) (cms)
00764> TOTAL HYD. 10:to205 1.92 .215 1.000 14.460 .000
00765> MAJOR SYST 01:20IOVL .00 .000 .000 .000 .000
00766> MINOR SYST 02:INFIL 1.92 .023 .800 14.494 .000
00767>
00768> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00770>
00771> Maximum MAJOR SYSTEM storage used = 122.(cu.m.)
00772>
00773>-----
00774> 001:0038-----
00775> CALIB NASHYD | Area (ha)= .09 Curve Number (CN)=32.00
00776> 07:EX3 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00777> U.H. Tp(hrs)= .500
00778>
00779> Unit Hyd Qpeak (cms)= .007
00781>
00782> PEAK FLOW (cms)= .000 (i)
00783> TIME TO PEAK (hrs)= 1.667
00784> RUNOFF VOLUME (mm)= 2.638
00785> TOTAL RAINFALL (mm)= 44.184
00786> RUNOFF COEFFICIENT = .060
00787>
00788> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00789>
00790>-----
00791> 001:0039-----
00792> CALIB NASHYD | Area (ha)= .05 Curve Number (CN)=55.00
00793> 04:206 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00794> U.H. Tp(hrs)= .250
00795>
00796> Unit Hyd Qpeak (cms)= .008
00797>
00798> PEAK FLOW (cms)= .001 (i)
00800> TIME TO PEAK (hrs)= 1.333
00801> RUNOFF VOLUME (mm)= 6.202
00802> TOTAL RAINFALL (mm)= 44.184
00803> RUNOFF COEFFICIENT = .140
00804>
00805> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00806>
00807>-----
00808> 001:0040-----
00809> ADD HYD (toDitch) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00810> (ha) (cms) (hrs) (mm) (cms)
00811> ID1 04:206 .05 .001 1.33 6.20 .000
00812> +ID2 07:EX3 .09 .000 1.67 2.64 .000
00813>-----
00814> SUM 10:toDitch .14 .001 1.42 3.91 .000
00815>
00816> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00817>
00818>-----
00819> 001:0041-----
00820> CHICAGO STORM | IDF curve parameters: A=1574.382
00821> Ptotal= 52.04 mm | B= 9.025
00822> C= .860
00823> used in: INTENSITY = A / (t + B)^C
00824>
00825> Duration of storm = 3.00 hrs
00826> Storm time step = 5.00 min
00827> Time to peak ratio = .33
00828>
00829>-----
00830> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00831> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00832> .08 3.493 | .63 25.826 | 1.58 11.834 | 2.33 4.773
00833> .17 3.869 | .92 61.712 | 1.67 10.197 | 2.42 4.475
00834> .25 4.337 | 1.00 162.470 | 1.75 8.945 | 2.50 4.212
00835> .33 4.937 | 1.08 79.037 | 1.83 7.960 | 2.58 3.979
00836> .42 5.731 | 1.17 44.466 | 1.92 7.166 | 2.67 3.770
00837> .50 6.830 | 1.25 29.772 | 2.00 6.514 | 2.75 3.583
00838> .58 8.439 | 1.33 21.949 | 2.08 5.970 | 2.83 3.414
00839> .67 10.998 | 1.42 17.198 | 2.17 5.510 | 2.92 3.261
00840> .75 15.601 | 1.50 14.051 | 2.25 5.115 | 3.00 3.121
00841>
00842>-----
00843> 001:0042-----
00844> CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
00845> 01:EX1 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00846> U.H. Tp(hrs)= .500
00847>
00848> Unit Hyd Qpeak (cms)= .079
00849>
00850> PEAK FLOW (cms)= .007 (i)
00851> TIME TO PEAK (hrs)= 1.667
00852> RUNOFF VOLUME (mm)= 3.771
00853> TOTAL RAINFALL (mm)= 52.043
00854> RUNOFF COEFFICIENT = .072
00855>
00856> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00857>
00858>-----
00859> 001:0043-----
00860> CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
00861> 02:EX2 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00862> U.H. Tp(hrs)= .500
00863>
00864> Unit Hyd Qpeak (cms)= .079
00865>
00866> PEAK FLOW (cms)= .007 (i)
00867> TIME TO PEAK (hrs)= 1.667
00868> RUNOFF VOLUME (mm)= 3.771
00869> TOTAL RAINFALL (mm)= 52.043
00870> RUNOFF COEFFICIENT = .072
00871>
00872> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00873>
00874>-----
00875> 001:0044-----
00876> CALIB STANDHYD | Area (ha)= .27
00877> 03:C201 DT= 1.00 | Total Imp(%)= 28.00 Dir. Conn.(%)= 5.00
00878>
00879> IMPERVIOUS PERVIOUS (i)
00880> Surface Area (ha)= .08 .19
00881> Dep. Storage (mm)= 2.00 5.00
00882> Average Slope (%)= 1.80 1.80
00883> Length (m)= 40.00 10.00
00884> Mannings n = .013 .025
00885>
00886>-----

00890> Max.eff.Inten.(mm/hr)= 162.47 44.95
00891> over (min) 1.00 2.00
00892> Storage Coeff. (min)= 1.02 (ii) 2.11 (iii)
00893> Unit Hyd. Tpeak (min)= 1.00 2.00
00894> Unit Hyd. peak (cms)= 1.06 .54
00895> *TOTALS*
00896> PEAK FLOW (cms)= .01 .02 .025 (iii)
00897> TIME TO PEAK (hrs)= 1.00 1.02 1.000
00898> RUNOFF VOLUME (mm)= 50.04 11.32 13.253
00899> TOTAL RAINFALL (mm)= 52.04 52.04 52.043
00900> RUNOFF COEFFICIENT = .96 .22 .255
00901>
00902> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00903> CN* = 55.0 Ia = Dep. Storage (Above)
00904> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00905> THAN THE STORAGE COEFFICIENT.
00906> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00907>
00908>-----
00909> 001:0045-----
00910> ADD HYD (20ltrench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00911> (ha) (cms) (hrs) (mm) (cms)
00912> ID1 01:EX1 1.04 .007 1.67 3.77 .000
00913> +ID2 03:C201 .27 .025 1.00 13.25 .000
00914>-----
00915> SUM 10:20ltrench 1.31 .025 1.00 5.72 .000
00916>
00917> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00918>
00919>-----
00920> 001:0046-----
00921> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
00922> TotalHyd 10:20ltre Number of inlets in system [NINLET] = 1
00923> Total minor system capacity = .001 (cms)
00924> Total major system storage [TMJSTO] = 108.(cu.m.)
00925>
00926> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00927> (ha) (cms) (hrs) (mm) (cms)
00928> TOTAL HYD. 10:20ltre 1.31 .025 1.000 5.725 .000
00929>-----
00930> MAJOR SYST 01:20IOVL .00 .000 .000 .000 .000
00931> MINOR SYST 03:INFIL 1.31 .001 .767 5.725 .000
00932>
00933> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00934>
00935> Maximum MAJOR SYSTEM storage used = 65.(cu.m.)
00936>
00937>-----
00938> 001:0047-----
00939> CALIB NASHYD | Area (ha)= .12 Curve Number (CN)=55.00
00940> 04:202 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00941> U.H. Tp(hrs)= .500
00942>
00943> Unit Hyd Qpeak (cms)= .009
00944>
00945> PEAK FLOW (cms)= .002 (i)
00946> TIME TO PEAK (hrs)= 1.667
00947> RUNOFF VOLUME (mm)= 8.676
00948> TOTAL RAINFALL (mm)= 52.043
00949> RUNOFF COEFFICIENT = .167
00950>
00951> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00952>
00953>-----
00954> 001:0048-----
00955> ADD HYD (202trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00956> (ha) (cms) (hrs) (mm) (cms)
00957> ID1 02:EX2 1.04 .007 1.67 3.77 .000
00958> +ID2 04:202 .12 .002 1.67 8.68 .000
00959>-----
00960> SUM 09:202trench 1.16 .009 1.67 4.28 .000
00961>
00962> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00963>
00964>-----
00965> 001:0049-----
00966> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
00967> TotalHyd 09:202tre Number of inlets in system [NINLET] = 1
00968> Total minor system capacity = .000 (cms)
00969> Total major system storage [TMJSTO] = 7.(cu.m.)
00970>
00971> ID: NHYD AREA QPEAK TPEAK R.V. DWF
00972> (ha) (cms) (hrs) (mm) (cms)
00973> TOTAL HYD. 09:202tre 1.16 .009 1.667 4.278 .000
00974>-----
00975> MAJOR SYST 02:20IOVL .89 .009 1.667 4.278 .000
00976> MINOR SYST 04:INFIL .27 .000 1.000 4.309 .000
00977>
00978> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00979>
00980> Maximum MAJOR SYSTEM storage used = 7.(cu.m.)
00981>
00982>-----
00983> 001:0050-----
00984> CALIB NASHYD | Area (ha)= .18 Curve Number (CN)=55.00
00985> 05:203 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
00986> U.H. Tp(hrs)= .500
00987>
00988> Unit Hyd Qpeak (cms)= .014
00989>
00990> PEAK FLOW (cms)= .003 (i)
00991> TIME TO PEAK (hrs)= 1.667
00992> RUNOFF VOLUME (mm)= 8.678
00993> TOTAL RAINFALL (mm)= 52.043
00994> RUNOFF COEFFICIENT = .167
00995>
00996> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00997>
00998>-----
00999> 001:0051-----
10000> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
10001> TotalHyd 05:203 Number of inlets in system [NINLET] = 1
10002> Total minor system capacity = .000 (cms)
10003> Total major system storage [TMJSTO] = 40.(cu.m.)
10004>
10005> ID: NHYD AREA QPEAK TPEAK R.V. DWF
10006> (ha) (cms) (hrs) (mm) (cms)
10007> TOTAL HYD. 05:203 .18 .003 1.667 8.678 .000
10008>-----
10009> MAJOR SYST 03:20IOVL .89 .000 .000 .000 .000
10010>
10011>-----

01017> MINOR SYST 04:INFIL .18 .000 1.083 8.730 .000
01018>
01019> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01020>
01021> Maximum MAJOR SYSTEM storage used = 12. (cu.m.)
01022>
01023>-----
01024> 001:0052-----
01025> | CALIB STANDHYD | Area (ha)= .31
01027> | 06:C204 DT= 1.00 | Total Imp(%)= 31.00 Dir. Conn.(%)= 5.00
01028>-----
01029> IMPERVIOUS PERVIOUS (i)
01030> Surface Area (ha)= .10 .21
01031> Dep. Storage (mm)= 2.00 5.00
01032> Average Slope (%)= 1.80 1.80
01033> Length (m)= 40.00 10.00
01034> Mannings n = .013 .025
01035>
01036> Max. eff. Inten. (mm/hr)= 162.47 48.93
01037> over (min) 1.00 2.00
01038> Storage Coeff. (min)= 1.02 (ii) 2.08 (ii)
01039> Unit Hyd. Tpeak (min)= 1.00 2.00
01040> Unit Hyd. peak (cms)= 1.06 .54
01041>
01042> PEAK FLOW (cms)= .01 .02
01043> TIME TO PEAK (hrs)= 1.00 1.02 1.000
01044> RUNOFF VOLUME (mm)= 50.04 11.76 13.671
01045> TOTAL RAINFALL (mm)= 52.04 52.04 52.043
01046> RUNOFF COEFFICIENT = .96 .23 .263
01047>
01048> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01049> CN* = 55.0 Ia = Dep. Storage (Above)
01050> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01051> THAN THE STORAGE COEFFICIENT.
01052> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01053>
01054>-----
01055> 001:0053-----
01056> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01058> | TotalHyd 06:C204 | Number of inlets in system [NINLET] = 1
01059> | | Total minor system capacity = .001 (cms)
01060> | | Total major system storage [TMJSTO] = 108. (cu.m.)
01061>-----
01062> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01063> (ha) (cms) (hrs) (mm) (cms)
01064> TOTAL HYD. 06:C204 .31 .030 1.000 13.671 .000
01065>-----
01066> MAJOR SYST 05:204OVL .00 .000 .000 .000 .000
01067> MINOR SYST 04:INFIL .31 .001 .750 13.680 .000
01068>
01069> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01070>
01071> Maximum MAJOR SYSTEM storage used = 35. (cu.m.)
01072>
01073>-----
01074> 001:0054-----
01075> | CALIB STANDHYD | Area (ha)= 1.13
01077> | 07:C205 DT= 1.00 | Total Imp(%)= 67.00 Dir. Conn.(%)= 35.00
01078>-----
01079> IMPERVIOUS PERVIOUS (i)
01080> Surface Area (ha)= .76 .37
01081> Dep. Storage (mm)= 2.00 5.00
01082> Average Slope (%)= 1.80 1.80
01083> Length (m)= 40.00 10.00
01084> Mannings n = .013 .025
01085>
01086> Max. eff. Inten. (mm/hr)= 162.47 97.11
01087> over (min) 1.00 2.00
01088> Storage Coeff. (min)= 1.02 (ii) 1.82 (ii)
01089> Unit Hyd. Tpeak (min)= 1.00 2.00
01090> Unit Hyd. peak (cms)= 1.06 .59
01091>
01092> PEAK FLOW (cms)= .18 .08
01093> TIME TO PEAK (hrs)= 1.00 1.02 1.000
01094> RUNOFF VOLUME (mm)= 50.04 15.81 27.791
01095> TOTAL RAINFALL (mm)= 52.04 52.04 52.043
01096> RUNOFF COEFFICIENT = .96 .30 .534
01097>
01098> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01099> CN* = 55.0 Ia = Dep. Storage (Above)
01100> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01101> THAN THE STORAGE COEFFICIENT.
01102> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01103>
01104>-----
01105> 001:0055-----
01106> | ADD HYD (to205) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01107> (ha) (cms) (hrs) (mm) (cms)
01108> ID1 01:201OVL .00 .000 .00 .00 .000
01109> +ID2 02:202OVL .89 .009 1.67 4.28 .000
01110> +ID3 03:203OVL .00 .000 .00 .00 .000
01111> +ID4 05:204OVL .00 .000 .00 .00 .000
01112> +ID5 07:C205 1.13 .261 1.00 27.79 .000
01113>-----
01114> SUM 10:to205 2.02 .261 1.00 17.41 .000
01115>
01116> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01117>
01118>-----
01119> 001:0056-----
01120> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .023 (cms)
01122> | TotalHyd 10:to205 | Number of inlets in system [NINLET] = 1
01123> | | Total minor system capacity = .023 (cms)
01124> | | Total major system storage [TMJSTO] = 761. (cu.m.)
01125>-----
01126> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01127> (ha) (cms) (hrs) (mm) (cms)
01128> TOTAL HYD. 10:to205 2.02 .261 1.000 17.407 .000
01129>-----
01130> MAJOR SYST 01:205OVL .00 .000 .000 .000 .000
01131> MINOR SYST 02:INFIL 2.02 .023 .767 17.434 .000
01132>
01133> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01134>
01135> Maximum MAJOR SYSTEM storage used = 178. (cu.m.)
01136>
01137>-----
01138> 001:0057-----
01139> | CALIB NASHYD | Area (ha)= .09 Curve Number (CN)=32.00
01140> | 07:EX3 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01141> | | U.H. Tp(hrs)= .500

01144> Unit Hyd Qpeak (cms)= .007
01145>
01146> PEAK FLOW (cms)= .001 (i)
01147> TIME TO PEAK (hrs)= 1.667
01148> RUNOFF VOLUME (mm)= 3.758
01149> TOTAL RAINFALL (mm)= 52.043
01150> RUNOFF COEFFICIENT = .072
01151>
01152> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01153>
01154>-----
01155> 001:0058-----
01156> | CALIB NASHYD | Area (ha)= .05 Curve Number (CN)=55.00
01157> | 04:206 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01158> | | U.H. Tp(hrs)= .250
01159>
01160> Unit Hyd Qpeak (cms)= .008
01161>
01162> PEAK FLOW (cms)= .001 (i)
01163> TIME TO PEAK (hrs)= 1.333
01164> RUNOFF VOLUME (mm)= 8.672
01165> TOTAL RAINFALL (mm)= 52.043
01166> RUNOFF COEFFICIENT = .167
01167>
01168> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01169>
01170>-----
01171> 001:0059-----
01172> | ADD HYD (toDitch) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01173> (ha) (cms) (hrs) (mm) (cms)
01174> ID1 04:206 .05 .001 1.33 8.67 .000
01175> +ID2 07:EX3 .09 .001 1.67 3.76 .000
01176>-----
01177> SUM 10:toDitch .14 .002 1.42 5.51 .000
01178>
01179> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01180>
01181>-----
01182> 001:0060-----
01183> | CHICAGO STORM | IDF curve parameters: A=2019.372
01184> | Ptotal= 61.48 mm | B= 9.824
01185> | | C= .875
01186> used in: INTENSITY = A / (t + B)^C
01187> Duration of storm = 3.00 hrs
01188> Storm time step = 5.00 min
01189> Time to peak ratio = .33
01190>
01191>-----
01192> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
01193> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
01194> .08 3.900 .83 31.054 1.58 13.926 2.33 5.406
01195> .17 4.339 .92 74.372 1.67 11.933 2.42 5.053
01196> .25 4.890 1.00 190.819 1.75 10.414 2.50 4.743
01197> .33 5.600 1.08 95.075 1.83 9.222 2.58 4.468
01198> .42 6.545 1.17 53.763 1.92 8.265 2.67 4.224
01199> .50 7.861 1.25 35.883 2.00 7.482 2.75 4.005
01200> .58 9.802 1.33 26.308 2.08 6.831 2.83 3.808
01201> .67 12.908 1.42 20.486 2.17 6.281 2.92 3.629
01202> .75 18.531 1.50 16.634 2.25 5.811 3.00 3.466
01203>
01204>-----
01205> 001:0061-----
01206> | CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
01207> | 01:EX1 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01208> | | U.H. Tp(hrs)= .500
01209>
01210> Unit Hyd Qpeak (cms)= .079
01211>
01212> PEAK FLOW (cms)= .011 (i)
01213> TIME TO PEAK (hrs)= 1.667
01214> RUNOFF VOLUME (mm)= 5.349
01215> TOTAL RAINFALL (mm)= 61.478
01216> RUNOFF COEFFICIENT = .087
01217>
01218> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01219>
01220>-----
01221> 001:0062-----
01222> | CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
01223> | 02:EX2 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01224> | | U.H. Tp(hrs)= .500
01225>
01226> Unit Hyd Qpeak (cms)= .079
01227>
01228> PEAK FLOW (cms)= .011 (i)
01229> TIME TO PEAK (hrs)= 1.667
01230> RUNOFF VOLUME (mm)= 5.349
01231> TOTAL RAINFALL (mm)= 61.478
01232> RUNOFF COEFFICIENT = .087
01233>
01234> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01235>
01236>-----
01237> 001:0063-----
01238> | CALIB STANDHYD | Area (ha)= .27
01239> | 03:C201 DT= 1.00 | Total Imp(%)= 28.00 Dir. Conn.(%)= 5.00
01240>-----
01241> IMPERVIOUS PERVIOUS (i)
01242> Surface Area (ha)= .08 .19
01243> Dep. Storage (mm)= 2.00 5.00
01244> Average Slope (%)= 1.80 1.80
01245> Length (m)= 40.00 10.00
01246> Mannings n = .013 .025
01247>
01248> Max. eff. Inten. (mm/hr)= 190.82 61.95
01249> over (min) 1.00 2.00
01250> Storage Coeff. (min)= .95 (ii) 1.92 (ii)
01251> Unit Hyd. Tpeak (min)= 1.00 2.00
01252> Unit Hyd. peak (cms)= 1.10 .57
01253>
01254> PEAK FLOW (cms)= .01 .03
01255> TIME TO PEAK (hrs)= 1.00 1.02 1.000
01256> RUNOFF VOLUME (mm)= 59.48 15.46 17.666
01257> TOTAL RAINFALL (mm)= 61.48 61.48 61.478
01258> RUNOFF COEFFICIENT = .97 .25 .287
01259>
01260> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01261> CN* = 55.0 Ia = Dep. Storage (Above)
01262> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01263> THAN THE STORAGE COEFFICIENT.

01271> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01272> -----
 01273> 001:0064
 01274> | ADD HYD (201trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01275> (ha) (cms) (hrs) (mm) (cms)
 01276> ID1 01:EX1 1.04 .011 1.67 5.35 .000
 01277> +ID2 03:C201 .27 .034 1.00 17.67 .000
 01278> =====
 01279> SUM 10:201trench 1.31 .035 1.00 7.89 .000
 01280> -----
 01281> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01282> -----
 01283> 001:0065
 01284> -----
 01285> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
 01286> | TotalHyd 10:201tre | Number of inlets in system [NINLET] = 1
 01287> | | Total minor system capacity = .001 (cms)
 01288> | | Total major system storage [TMJSTO] = 108.(cu.m.)
 01289> -----
 01290> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01291> (ha) (cms) (hrs) (mm) (cms)
 01292> TOTAL HYD. 10:201tre 1.31 .035 1.000 7.887 .000
 01293> =====
 01294> MAJOR SYST 01:201OVL .00 .000 .000 .000 .000
 01295> MINOR SYST 03:INFIL 1.31 .001 .717 7.891 .000
 01296> -----
 01297> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01298> -----
 01299> Maximum MAJOR SYSTEM storage used = 93.(cu.m.)
 01300> -----
 01301> 001:0066
 01302> | CALIB NASHYD | Area (ha)= .12 Curve Number (CN)=55.00
 01303> | 04:202 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01304> | | U.H. Tp(hrs)= .500
 01305> -----
 01306> Unit Hyd Qpeak (cms)= .009
 01307> -----
 01308> PEAK FLOW (cms)= .003 (i)
 01309> TIME TO PEAK (hrs)= 1.667
 01310> RUNOFF VOLUME (mm)= 12.060
 01311> TOTAL RAINFALL (mm)= 61.478
 01312> RUNOFF COEFFICIENT = .196
 01313> -----
 01314> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01315> -----
 01316> 001:0067
 01317> | ADD HYD (202trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01318> (ha) (cms) (hrs) (mm) (cms)
 01319> ID1 02:EX2 1.04 .011 1.67 5.35 .000
 01320> +ID2 04:202 .12 .003 1.67 12.06 .000
 01321> =====
 01322> SUM 09:202trench 1.16 .013 1.67 6.04 .000
 01323> -----
 01324> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01325> -----
 01326> 001:0068
 01327> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
 01328> | TotalHyd 09:202tre | Number of inlets in system [NINLET] = 1
 01329> | | Total minor system capacity = .000 (cms)
 01330> | | Total major system storage [TMJSTO] = 7.(cu.m.)
 01331> -----
 01332> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01333> (ha) (cms) (hrs) (mm) (cms)
 01334> TOTAL HYD. 09:202tre 1.16 .013 1.667 6.043 .000
 01335> =====
 01336> MAJOR SYST 02:202OVL .97 .013 1.667 6.043 .000
 01337> MINOR SYST 04:INFIL .19 .000 1.000 6.084 .000
 01338> -----
 01339> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01340> -----
 01341> Maximum MAJOR SYSTEM storage used = 7.(cu.m.)
 01342> -----
 01343> 001:0069
 01344> | CALIB NASHYD | Area (ha)= .18 Curve Number (CN)=55.00
 01345> | 05:203 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01346> | | U.H. Tp(hrs)= .500
 01347> -----
 01348> Unit Hyd Qpeak (cms)= .014
 01349> -----
 01350> PEAK FLOW (cms)= .004 (i)
 01351> TIME TO PEAK (hrs)= 1.667
 01352> RUNOFF VOLUME (mm)= 12.064
 01353> TOTAL RAINFALL (mm)= 61.478
 01354> RUNOFF COEFFICIENT = .196
 01355> -----
 01356> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01357> -----
 01358> 001:0070
 01359> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
 01360> | TotalHyd 05:203 | Number of inlets in system [NINLET] = 1
 01361> | | Total minor system capacity = .000 (cms)
 01362> | | Total major system storage [TMJSTO] = 40.(cu.m.)
 01363> -----
 01364> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01365> (ha) (cms) (hrs) (mm) (cms)
 01366> TOTAL HYD. 05:203 .18 .004 1.667 12.064 .000
 01367> =====
 01368> MAJOR SYST 03:203OVL .00 .000 .000 .000 .000
 01369> MINOR SYST 04:INFIL .18 .000 1.083 12.077 .000
 01370> -----
 01371> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01372> -----
 01373> Maximum MAJOR SYSTEM storage used = 18.(cu.m.)
 01374> -----
 01375> 001:0071
 01376> | CALIB STANDHYD | Area (ha)= .31
 01377> | 06:C204 DT= 1.00 | Total Imp(%)= 31.00 Dir. Conn.(%)= 5.00
 01378> -----
 01379> IMPERVIOUS PERVIOUS (i)
 01380> Surface Area (ha)= 2.10 .21
 01381> Dep. Storage (mm)= 1.00 5.00
 01382> Average Slope (%)= 1.80 1.80

01398> Length (m)= 40.00 10.00
 01399> Mannings n = .013 .025
 01400> -----
 01401> Max.eff.Inten.(mm/hr)= 190.82 67.24
 01402> over (min) 1.00 2.00
 01403> Storage Coeff. (min)= .95 (ii) 1.89 (ii)
 01404> Unit Hyd. Tpeak (min)= 1.00 2.00
 01405> Unit Hyd. peak (cms)= 1.10 .58
 01406> -----
 01407> PEAK FLOW (cms)= .01 .03 *TOTALS*
 01408> TIME TO PEAK (hrs)= 1.00 1.02 1.000
 01409> RUNOFF VOLUME (mm)= 59.48 16.03 18.199
 01410> TOTAL RAINFALL (mm)= 61.48 61.48 61.478
 01411> RUNOFF COEFFICIENT = .97 .26 .296
 01412> -----
 01413> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01414> CN* = 55.0 Ia = Dep. Storage (Above)
 01415> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01416> THAN THE STORAGE COEFFICIENT.
 01417> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01418> -----
 01419> 001:0072
 01420> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
 01421> | TotalHyd 06:C204 | Number of inlets in system [NINLET] = 1
 01422> | | Total minor system capacity = .001 (cms)
 01423> | | Total major system storage [TMJSTO] = 108.(cu.m.)
 01424> -----
 01425> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01426> (ha) (cms) (hrs) (mm) (cms)
 01427> TOTAL HYD. 06:C204 .31 .041 1.000 18.199 .000
 01428> =====
 01429> MAJOR SYST 05:204OVL .00 .000 .000 .000 .000
 01430> MINOR SYST 04:INFIL .31 .001 .700 18.202 .000
 01431> -----
 01432> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01433> -----
 01434> Maximum MAJOR SYSTEM storage used = 49.(cu.m.)
 01435> -----
 01436> 001:0073
 01437> | CALIB STANDHYD | Area (ha)= 1.13
 01438> | 07:C205 DT= 1.00 | Total Imp(%)= 67.00 Dir. Conn.(%)= 35.00
 01439> -----
 01440> IMPERVIOUS PERVIOUS (i)
 01441> Surface Area (ha)= .76 .37
 01442> Dep. Storage (mm)= 2.00 5.00
 01443> Average Slope (%)= 1.80 1.80
 01444> Length (m)= 40.00 10.00
 01445> Mannings n = .013 .025
 01446> -----
 01447> Max.eff.Inten.(mm/hr)= 190.82 130.68
 01448> over (min) 1.00 2.00
 01449> Storage Coeff. (min)= .95 (ii) 1.67 (ii)
 01450> Unit Hyd. Tpeak (min)= 1.00 2.00
 01451> Unit Hyd. peak (cms)= 1.10 .62
 01452> -----
 01453> PEAK FLOW (cms)= .21 .12 *TOTALS*
 01454> TIME TO PEAK (hrs)= 1.00 1.00 1.000
 01455> RUNOFF VOLUME (mm)= 59.48 21.12 34.548
 01456> TOTAL RAINFALL (mm)= 61.48 61.48 61.478
 01457> RUNOFF COEFFICIENT = .97 .34 .562
 01458> -----
 01459> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01460> CN* = 55.0 Ia = Dep. Storage (Above)
 01461> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01462> THAN THE STORAGE COEFFICIENT.
 01463> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01464> -----
 01465> 001:0074
 01466> | ADD HYD (to205) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01467> (ha) (cms) (hrs) (mm) (cms)
 01468> ID1 01:201OVL .00 .000 .000 .00 .000
 01469> +ID2 02:202OVL .97 .013 1.67 6.04 .000
 01470> +ID3 03:203OVL .00 .000 .00 .00 .000
 01471> +ID4 05:204OVL .00 .000 .00 .00 .000
 01472> +ID5 07:C205 1.13 .324 1.00 34.55 .000
 01473> =====
 01474> SUM 10:to205 2.10 .324 1.00 21.39 .000
 01475> -----
 01476> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01477> -----
 01478> 001:0075
 01479> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .023 (cms)
 01480> | TotalHyd 10:to205 | Number of inlets in system [NINLET] = 1
 01481> | | Total minor system capacity = .023 (cms)
 01482> | | Total major system storage [TMJSTO] = 761.(cu.m.)
 01483> -----
 01484> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01485> (ha) (cms) (hrs) (mm) (cms)
 01486> TOTAL HYD. 10:to205 2.10 .324 1.000 21.386 .000
 01487> =====
 01488> MAJOR SYST 01:205OVL .00 .000 .000 .000 .000
 01489> MINOR SYST 02:INFIL 2.10 .023 .767 21.410 .000
 01490> -----
 01491> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01492> -----
 01493> Maximum MAJOR SYSTEM storage used = 257.(cu.m.)
 01494> -----
 01495> 001:0076
 01496> | CALIB NASHYD | Area (ha)= .09 Curve Number (CN)=32.00
 01497> | 07:EX3 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01498> | | U.H. Tp(hrs)= .500
 01499> -----
 01500> Unit Hyd Qpeak (cms)= .007
 01501> -----
 01502> PEAK FLOW (cms)= .001 (i)
 01503> TIME TO PEAK (hrs)= 1.667
 01504> RUNOFF VOLUME (mm)= 5.339
 01505> TOTAL RAINFALL (mm)= 61.478
 01506> RUNOFF COEFFICIENT = .087
 01507> -----
 01508> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01509> -----
 01510> 001:0077
 01511> | CALIB NASHYD | Area (ha)= .05 Curve Number (CN)=55.00
 01512> | 04:206 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01513> -----
 01514> Unit Hyd Qpeak (cms)= .007
 01515> -----
 01516> PEAK FLOW (cms)= .001 (i)
 01517> TIME TO PEAK (hrs)= 1.667
 01518> RUNOFF VOLUME (mm)= 5.339
 01519> TOTAL RAINFALL (mm)= 61.478
 01520> RUNOFF COEFFICIENT = .087

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01525>----- U.H. Tp(hrs)= .250
01526>
01527> Unit Hyd Qpeak (cms)= .008
01528>
01529> PEAK FLOW (cms)= .002 (i)
01530> TIME TO PEAK (hrs)= 1.333
01531> RUNOFF VOLUME (mm)= 12.055
01532> TOTAL RAINFALL (mm)= 61.478
01533> RUNOFF COEFFICIENT = .196
01534>
01535> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01536>
01537>-----
01538> 001:0078-----
01539>
01540> | ADD HYD (toDitch ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01541> | | (ha) (cms) (hrs) (mm) (cms)
01542> | ID1 04:206 | .05 .002 1.33 12.05 .000
01543> | +ID2 07:EX3 | .09 .001 1.67 5.34 .000
01544> | | | | | | |
01545> | SUM 10:toDitch | .14 .002 1.42 7.74 .000
01546>
01547> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01548>
01549>-----
01550> 001:0079-----
01551>
01552> | CHICAGO STORM | IDP curve parameters: A=2270.665
01553> | Ptotal= 68.72 mm | B= 9.984
01554> | | C= .876
01555> | |
01556> | used in: INTENSITY = A / (t + B)^C
01557> | Duration of storm = 3.00 hrs
01558> | Storm time step = 5.00 min
01559> | Time to peak ratio = .33
01560>
01561> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
01562> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
01563> .08 4.355 .83 34.855 1.58 15.620 2.33 6.045
01564> .17 4.848 .92 83.238 1.67 13.380 2.42 5.648
01565> .25 5.466 1.00 211.984 1.75 11.672 2.50 5.300
01566> .33 6.262 1.08 106.298 1.83 10.332 2.58 4.992
01567> .42 7.324 1.17 60.284 1.92 9.257 2.67 4.718
01568> .50 8.802 1.25 40.273 2.00 8.377 2.75 4.473
01569> .58 10.984 1.33 29.532 2.08 7.645 2.83 4.251
01570> .67 14.476 1.42 22.993 2.17 7.027 2.92 4.051
01571> .75 20.797 1.50 18.664 2.25 6.500 3.00 3.869
01572>
01573>-----
01574> 001:0080-----
01575>
01576> | CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
01577> | 01:EX1 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01578> | U.H. Tp(hrs)= .500
01579>
01580> Unit Hyd Qpeak (cms)= .079
01581>
01582> PEAK FLOW (cms)= .013 (i)
01583> TIME TO PEAK (hrs)= 1.667
01584> RUNOFF VOLUME (mm)= 6.727
01585> TOTAL RAINFALL (mm)= 68.716
01586> RUNOFF COEFFICIENT = .098
01587>
01588> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01589>
01590>-----
01591> 001:0081-----
01592>
01593> | CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
01594> | 02:EX2 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01595> | U.H. Tp(hrs)= .500
01596>
01597> Unit Hyd Qpeak (cms)= .079
01598>
01599> PEAK FLOW (cms)= .013 (i)
01600> TIME TO PEAK (hrs)= 1.667
01601> RUNOFF VOLUME (mm)= 6.727
01602> TOTAL RAINFALL (mm)= 68.716
01603> RUNOFF COEFFICIENT = .098
01604>
01605> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01606>
01607>-----
01608> 001:0082-----
01609>
01610> | CALIB STANDHYD | Area (ha)= .27
01611> | 03:C201 DT= 1.00 | Total Imp(%)= 28.00 Dir. Conn.(%)= 5.00
01612>
01613> IMPERVIOUS PERVIOUS (i)
01614> Surface Area (ha)= .10 .10
01615> Dep. Storage (mm)= 2.00 5.00
01616> Average Slope (%)= 1.80 1.80
01617> Length (m)= 40.00 10.00
01618> Mannings n = .013 .025
01619>
01620> Max.eff.Inten.(mm/hr)= 211.98 76.08
01621> over (min)= 1.00 2.00
01622> Storage Coeff. (min)= .92 (ii) 1.80 (ii)
01623> Unit Hyd. Tpeak (min)= 1.00 2.00
01624> Unit Hyd. peak (cms)= 1.13 .60
01625>
01626> PEAK FLOW (cms)= .01 .03 *TOTALS*
01627> TIME TO PEAK (hrs)= 1.00 1.02 .042 (iii)
01628> RUNOFF VOLUME (mm)= 66.71 18.95 21.340
01629> TOTAL RAINFALL (mm)= 68.72 68.72 68.716
01630> RUNOFF COEFFICIENT = .97 .28 .311
01631>
01632> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01633> CN* = 55.0 Ia = Dep. Storage (Above)
01634> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01635> THAN THE STORAGE COEFFICIENT.
01636> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01637>
01638>-----
01639> 001:0083-----
01640>
01641> | ADD HYD (201trench ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01642> | | (ha) (cms) (hrs) (mm) (cms)
01643> | ID1 01:EX1 | 1.04 .013 1.67 6.73 .000
01644> | +ID2 03:C201 | .27 .042 1.00 21.34 .000
01645> | | | | | | |
01646> | SUM 10:201trench | 1.31 .043 1.00 9.74 .000
01647>
01648> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01649>
01650>-----
01651> 001:0084-----

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01652>-----
01653> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
01654> TotalHyd 10:201tre | Number of inlets in system [NINLET] = 1
01655> | Total minor system capacity = .001 (cms)
01656> | Total major system storage [TMJSTO] = 108.(cu.m.)
01657>
01658> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01659> (ha) (cms) (hrs) (mm) (cms)
01660> TOTAL HYD. 10:201tre 1.31 .043 1.000 9.738 .000
01661> =====
01662> MAJOR SYST 01:201OVL .09 .006 2.817 9.738 .000
01663> MINOR SYST 03:INFLL 1.22 .001 .683 9.739 .000
01664>
01665> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01666>
01667> Maximum MAJOR SYSTEM storage used = 108.(cu.m.)
01668>-----
01669> 001:0085-----
01670>
01671> CALIB NASHYD | Area (ha)= .12 Curve Number (CN)=55.00
01672> | 04:202 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01673> | U.H. Tp(hrs)= .500
01674>
01675> Unit Hyd Qpeak (cms)= .009
01676>
01677> PEAK FLOW (cms)= .003 (i)
01678> TIME TO PEAK (hrs)= 1.667
01679> RUNOFF VOLUME (mm)= 14.943
01680> TOTAL RAINFALL (mm)= 68.716
01681> RUNOFF COEFFICIENT = .217
01682>
01683> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01684>
01685>-----
01686> 001:0086-----
01687>
01688> ADD HYD (202trench ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01689> | | (ha) (cms) (hrs) (mm) (cms)
01690> | ID1 02:EX2 | 1.04 .013 1.67 6.73 .000
01691> | +ID2 04:202 | .12 .003 1.67 14.94 .000
01692> | | | | | | |
01693> | SUM 09:202trench | 1.16 .017 1.67 7.58 .000
01694>
01695> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01696>
01697>-----
01698> 001:0087-----
01699>
01700> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
01701> TotalHyd 09:202tre | Number of inlets in system [NINLET] = 1
01702> | Total minor system capacity = .000 (cms)
01703> | Total major system storage [TMJSTO] = 7.(cu.m.)
01704>
01705> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01706> (ha) (cms) (hrs) (mm) (cms)
01707> TOTAL HYD. 09:202tre 1.16 .017 1.667 7.577 .000
01708> =====
01709> MAJOR SYST 02:202OVL 1.01 .016 1.667 7.577 .000
01710> MINOR SYST 04:INFLL .15 .000 1.000 7.638 .000
01711>
01712> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01713>
01714> Maximum MAJOR SYSTEM storage used = 7.(cu.m.)
01715>-----
01716> 001:0088-----
01717>
01718> CALIB NASHYD | Area (ha)= .18 Curve Number (CN)=55.00
01719> | 05:203 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
01720> | U.H. Tp(hrs)= .500
01721>
01722> Unit Hyd Qpeak (cms)= .014
01723>
01724> PEAK FLOW (cms)= .005 (i)
01725> TIME TO PEAK (hrs)= 1.667
01726> RUNOFF VOLUME (mm)= 14.947
01727> TOTAL RAINFALL (mm)= 68.716
01728> RUNOFF COEFFICIENT = .218
01729>
01730> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01731>
01732>-----
01733> 001:0089-----
01734>
01735> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
01736> TotalHyd 05:203 | Number of inlets in system [NINLET] = 1
01737> | Total minor system capacity = .000 (cms)
01738> | Total major system storage [TMJSTO] = 40.(cu.m.)
01739>
01740> ID: NHYD AREA QPEAK TPEAK R.V. DWF
01741> (ha) (cms) (hrs) (mm) (cms)
01742> TOTAL HYD. 05:203 .18 .005 1.667 14.947 .000
01743> =====
01744> MAJOR SYST 03:203OVL .00 .000 .000 .000 .000
01745> MINOR SYST 04:INFLL .18 .000 1.000 14.947 .000
01746>
01747> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01748>
01749> Maximum MAJOR SYSTEM storage used = 23.(cu.m.)
01750>-----
01751> 001:0090-----
01752>
01753> CALIB STANDHYD | Area (ha)= .31
01754> | 06:C204 DT= 1.00 | Total Imp(%)= 31.00 Dir. Conn.(%)= 5.00
01755>
01756> IMPERVIOUS PERVIOUS (i)
01757> Surface Area (ha)= .10 .21
01758> Dep. Storage (mm)= 2.00 5.00
01759> Average Slope (%)= 1.80 1.80
01760> Length (m)= 40.00 10.00
01761> Mannings n = .013 .025
01762>
01763> Max.eff.Inten.(mm/hr)= 211.98 82.44
01764> over (min)= 1.00 2.00
01765> Storage Coeff. (min)= .92 (ii) 1.80 (ii)
01766> Unit Hyd. Tpeak (min)= 1.00 2.00
01767> Unit Hyd. peak (cms)= 1.13 .60
01768>
01769> PEAK FLOW (cms)= .01 .04 *TOTALS*
01770> TIME TO PEAK (hrs)= 1.00 1.02 .050 (iii)
01771> RUNOFF VOLUME (mm)= 66.71 18.95 21.340
01772> TOTAL RAINFALL (mm)= 68.72 68.72 68.716
01773> RUNOFF COEFFICIENT = .97 .29 .320
01774>
01775> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01776>
01777>-----

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01779> CN* = 55.0 Ia = Dep. Storage (Above)
 01780> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01781> THAN THE STORAGE COEFFICIENT.
 01782> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01783>
 01784>
 01785> 001:0091-----
 01786> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
 01787> | TotalHyd 06:C204 | Number of inlets in system [NINLET] = 1
 01788> | | Total minor system capacity = .001 (cms)
 01789> | | Total major system storage [TMJSTO] = 108.(cu.m.)
 01790>
 01791> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01792> (ha) (cms) (hrs) (mm) (cms)
 01793> TOTAL HYD. 06:C204 .31 .050 1.000 21.964 .000
 01794> MAJOR SYST 05:204OVL .00 .000 .000 .000 .000
 01795> MINOR SYST 04:INFIL .31 .001 .683 21.978 .000
 01796>
 01797> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01798>
 01799> Maximum MAJOR SYSTEM storage used = 60.(cu.m.)
 01800>
 01801>
 01802> 001:0092-----
 01803> CALIB STANDHYD | Area (ha)= 1.13
 01804> | 07:C205 DT= 1.00 | Total Imp(%)= 67.00 Dir. Conn.(%)= 35.00
 01805>
 01806> IMPERVIOUS PERVIOUS (i)
 01807> Surface Area (ha)= .76 .37
 01808> Dep. Storage (mm)= 2.00 5.00
 01809> Average Slope (%)= 1.80 1.80
 01810> Length (m)= 40.00 10.00
 01811> Mannings n = .013 .025
 01812> Max.eff.Inten.(mm/hr)= 211.98 158.06
 01813> over (min) 1.00 2.00
 01814> Storage Coeff. (min)= .92 (ii) 1.58 (ii)
 01815> Unit Hyd. Tpeak (min)= 1.00 2.00
 01816> Unit Hyd. peak (cms)= 1.13 .65
 01817>
 01818> *TOTALS*
 01819> PEAK FLOW (cms)= .23 .14
 01820> TIME TO PEAK (hrs)= 1.00 1.00
 01821> RUNOFF VOLUME (mm)= 66.72 25.51 39.931
 01822> TOTAL RAINFALL (mm)= 68.72 68.72 68.716
 01823> RUNOFF COEFFICIENT = .97 .37
 01824>
 01825> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01826> CN* = 55.0 Ia = Dep. Storage (Above)
 01827> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01828> THAN THE STORAGE COEFFICIENT.
 01829> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01830>
 01831>
 01832> 001:0093-----
 01833> ADD HYD (to205) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01834> (ha) (cms) (hrs) (mm) (cms)
 01835> ID1 01:201OVL .09 .006 2.82 9.74 .000
 01836> +ID2 02:202OVL 1.01 .016 1.67 7.58 .000
 01837> +ID3 03:203OVL .00 .000 .000 .00 .000
 01838> +ID4 05:204OVL .00 .000 .000 .00 .000
 01839> +ID5 07:C205 1.13 .374 1.00 39.93 .000
 01840> SUM 10:to205 2.22 .374 1.00 24.10 .000
 01841>
 01842> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01843>
 01844>
 01845> 001:0094-----
 01846> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .023 (cms)
 01847> | TotalHyd 10:to205 | Number of inlets in system [NINLET] = 1
 01848> | | Total minor system capacity = .023 (cms)
 01849> | | Total major system storage [TMJSTO] = 761.(cu.m.)
 01850>
 01851> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 01852> (ha) (cms) (hrs) (mm) (cms)
 01853> TOTAL HYD. 10:to205 2.22 .374 1.000 24.101 .000
 01854> MAJOR SYST 01:205OVL .00 .000 .000 .000 .000
 01855> MINOR SYST 02:INFIL 2.22 .023 .700 24.146 .000
 01856>
 01857> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01858>
 01859> Maximum MAJOR SYSTEM storage used = 322.(cu.m.)
 01860>
 01861>
 01862> 001:0095-----
 01863> CALIB NASHYD | Area (ha)= .09 Curve Number (CN)=32.00
 01864> | 07:EX3 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01865> | | U.H. Tp(hrs)= 5.00
 01866>
 01867> Unit Hyd Qpeak (cms)= .007
 01868>
 01869> PEAK FLOW (cms)= .001 (i)
 01870> TIME TO PEAK (hrs)= 1.667
 01871> RUNOFF VOLUME (mm)= 6.714
 01872> TOTAL RAINFALL (mm)= 68.716
 01873> RUNOFF COEFFICIENT = .098
 01874>
 01875> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01876>
 01877>
 01878> 001:0096-----
 01879> CALIB NASHYD | Area (ha)= .05 Curve Number (CN)=55.00
 01880> | 04:206 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01881> | | U.H. Tp(hrs)= .250
 01882>
 01883> Unit Hyd Qpeak (cms)= .008
 01884>
 01885> PEAK FLOW (cms)= .002 (i)
 01886> TIME TO PEAK (hrs)= 1.333
 01887> RUNOFF VOLUME (mm)= 14.941
 01888> TOTAL RAINFALL (mm)= 68.716
 01889> RUNOFF COEFFICIENT = .217
 01890>
 01891> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01892>
 01893>
 01894> 001:0097-----
 01895> ADD HYD (toDitch) | ID: NHYD AREA QPEAK TPEAK R.V. DWF

01906>----- (ha) (cms) (hrs) (mm) (cms)
 01907> ID1 04:206 .05 .002 1.33 14.94 .000
 01908> +ID2 07:EX3 .09 .001 1.67 6.71 .000
 01909>-----
 01910> SUM 10:toDitch .14 .003 1.42 9.65 .000
 01911>
 01912> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 01913>
 01914>
 01915> 001:0098-----
 01916> CHICAGO STORM | IDF curve parameters: A=2619.363
 01917> | Ptotal= 75.83 mm | B= 10.500
 01918> | | C= .884
 01919> used in: INTENSITY = A / (t + B)^C
 01920>
 01921> Duration of storm = 3.00 hrs
 01922> Storm time step = 5.00 min
 01923> Time to peak ratio = .33
 01924>
 01925> TIME RAIN TIME RAIN TIME RAIN TIME RAIN
 01926> hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
 01927> .08 4.669 .83 38.909 1.58 17.257 2.33 6.536
 01928> .17 5.212 .92 92.768 1.67 14.738 2.42 6.097
 01929> .25 5.895 1.00 232.243 1.75 12.820 2.50 5.712
 01930> .33 6.777 1.08 118.260 1.83 11.318 2.58 5.372
 01931> .42 7.958 1.17 67.393 1.92 10.115 2.67 5.069
 01932> .50 9.607 1.25 45.001 2.00 9.132 2.75 4.799
 01933> .58 12.049 1.33 32.924 2.08 8.315 2.83 4.555
 01934> .67 15.971 1.42 25.560 2.17 7.627 2.92 4.335
 01935> .75 23.085 1.50 20.683 2.25 7.041 3.00 4.135
 01936>
 01937>
 01938> 001:0099-----
 01939> CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
 01940> | 01:EX1 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01941> | | U.H. Tp(hrs)= .500
 01942>
 01943> Unit Hyd Qpeak (cms)= .079
 01944>
 01945> PEAK FLOW (cms)= .016 (i)
 01946> TIME TO PEAK (hrs)= 1.667
 01947> RUNOFF VOLUME (mm)= 8.215
 01948> TOTAL RAINFALL (mm)= 75.828
 01949> RUNOFF COEFFICIENT = .108
 01950>
 01951> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01952>
 01953>
 01954> 001:0100-----
 01955> CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
 01956> | 02:EX2 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
 01957> | | U.H. Tp(hrs)= .500
 01958>
 01959> Unit Hyd Qpeak (cms)= .079
 01960>
 01961> PEAK FLOW (cms)= .016 (i)
 01962> TIME TO PEAK (hrs)= 1.667
 01963> RUNOFF VOLUME (mm)= 8.215
 01964> TOTAL RAINFALL (mm)= 75.828
 01965> RUNOFF COEFFICIENT = .108
 01966>
 01967> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01968>
 01969>
 01970> 001:0101-----
 01971> CALIB STANDHYD | Area (ha)= .27
 01972> | 03:C201 DT= 1.00 | Total Imp(%)= 28.00 Dir. Conn.(%)= 5.00
 01973>
 01974> IMPERVIOUS PERVIOUS (i)
 01975> Surface Area (ha)= .08 .19
 01976> Dep. Storage (mm)= 2.00 5.00
 01977> Average Slope (%)= 1.80 1.80
 01978> Length (m)= 40.00 10.00
 01979> Mannings n = .013 .025
 01980> Max.eff.Inten.(mm/hr)= 232.24 90.74
 01981> over (min) 1.00 2.00
 01982> Storage Coeff. (min)= .88 (ii) 1.71 (ii)
 01983> Unit Hyd. Tpeak (min)= 1.00 2.00
 01984> Unit Hyd. peak (cms)= 1.15 .62
 01985>
 01986> *TOTALS*
 01987> PEAK FLOW (cms)= .01 .04
 01988> TIME TO PEAK (hrs)= 1.00 1.02
 01989> RUNOFF VOLUME (mm)= 73.83 22.61
 01990> TOTAL RAINFALL (mm)= 75.83 75.83
 01991> RUNOFF COEFFICIENT = .97 .30
 01992>
 01993> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 01994> CN* = 55.0 Ia = Dep. Storage (Above)
 01995> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 01996> THAN THE STORAGE COEFFICIENT.
 01997> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 01998>
 01999>
 02000>
 02001>
 02002>
 02003> 001:0102-----
 02004> ADD HYD (201trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
 02005> (ha) (cms) (hrs) (mm) (cms)
 02006> ID1 01:EX1 1.04 .016 1.67 8.22 .000
 02007> +ID2 03:C201 .27 .050 1.00 25.17 .000
 02008> SUM 10:201trench 1.31 .051 1.00 11.71 .000
 02009>
 02010> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 02011>
 02012>
 02013>
 02014> 001:0103-----
 02015> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
 02016> | TotalHyd 10:201tre | Number of inlets in system [NINLET] = 1
 02017> | | Total minor system capacity = .001 (cms)
 02018> | | Total major system storage [TMJSTO] = 108.(cu.m.)
 02019>
 02020> ID: NHYD AREA QPEAK TPEAK R.V. DWF
 02021> (ha) (cms) (hrs) (mm) (cms)
 02022> TOTAL HYD. 10:201tre 1.31 .051 1.000 11.709 .000
 02023> MAJOR SYST 01:201OVL .29 .014 2.183 11.709 .000
 02024> MINOR SYST 03:INFIL 1.02 .001 .683 11.713 .000
 02025>
 02026> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 02027>
 02028> Maximum MAJOR SYSTEM storage used = 108.(cu.m.)
 02029>
 02030>
 02031>
 02032>

02033>-----
02034>-----
02035> 001:0104-----
02036>-----
02037> | CALIB NASHYD | Area (ha)= .12 Curve Number (CN)=55.00
02038> | 04:202 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02039> | U.H. Tp(hrs)= .500
02040>-----
02041> Unit Hyd Qpeak (cms)= .009
02042>-----
02043> PEAK FLOW (cms)= .004 (i)
02044> TIME TO PEAK (hrs)= 1.667
02045> RUNOFF VOLUME (mm)= 17.997
02046> TOTAL RAINFALL (mm)= 75.828
02047> RUNOFF COEFFICIENT = .237
02048>-----
02049> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02050>-----
02051>-----
02052> 001:0105-----
02053>-----
02054> | ADD HYD (202:trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02055> | (ha) (cms) (hrs) (mm) (cms)
02056> | ID1 02:EX2 1.04 .016 1.67 8.22 .000
02057> | +ID2 04:202 .12 .004 1.67 18.00 .000
02058> |=====|
02059> | SUM 09:202:trench 1.16 .021 1.67 9.23 .000
02060>-----
02061> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02062>-----
02063>-----
02064> 001:0106-----
02065>-----
02066> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
02067> | TotalHyd 09:202:tre | Number of inlets in system [NINLET] = 1
02068> | Total minor system capacity = .000 (cms)
02069> | Total major system storage [TMJSTO] = 7.(cu.m.)
02070>-----
02071> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02072> | (ha) (cms) (hrs) (mm) (cms)
02073> | TOTAL HYD. 09:202:tre 1.16 .021 1.667 9.227 .000
02074>-----
02075> MAJOR SYST 02:202OVL 1.03 .020 1.667 9.227 .000
02076> MINOR SYST 04:INFIL .13 .000 1.000 9.250 .000
02077>-----
02078> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02079>-----
02080> Maximum MAJOR SYSTEM storage used = 7.(cu.m.)
02081>-----
02082>-----
02083> 001:0107-----
02084>-----
02085> | CALIB NASHYD | Area (ha)= .18 Curve Number (CN)=55.00
02086> | 05:203 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02087> | U.H. Tp(hrs)= .500
02088>-----
02089> Unit Hyd Qpeak (cms)= .014
02090>-----
02091> PEAK FLOW (cms)= .006 (i)
02092> TIME TO PEAK (hrs)= 1.667
02093> RUNOFF VOLUME (mm)= 17.999
02094> TOTAL RAINFALL (mm)= 75.828
02095> RUNOFF COEFFICIENT = .237
02096>-----
02097> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02098>-----
02099>-----
02100> 001:0108-----
02101>-----
02102> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
02103> | TotalHyd 05:203 | Number of inlets in system [NINLET] = 1
02104> | Total minor system capacity = .000 (cms)
02105> | Total major system storage [TMJSTO] = 40.(cu.m.)
02106>-----
02107> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02108> | (ha) (cms) (hrs) (mm) (cms)
02109> | TOTAL HYD. 05:203 .18 .006 1.667 17.999 .000
02110>-----
02111> MAJOR SYST 03:203OVL .00 .000 .000 .000 .000
02112> MINOR SYST 04:INFIL .18 .000 1.000 18.044 .000
02113>-----
02114> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02115>-----
02116> Maximum MAJOR SYSTEM storage used = 28.(cu.m.)
02117>-----
02118>-----
02119> 001:0109-----
02120>-----
02121> | CALIB STANDHYD | Area (ha)= .31
02122> | 06:C204 DT= 1.00 | Total Imp(%)= 31.00 Dir. Conn.(%)= 5.00
02123>-----
02124> IMPERVIOUS PERVIOUS (i)
02125> | Surface Area (ha)= .10 .21
02126> | Dep. Storage (mm)= 2.00 5.00
02127> | Average Slope (%)= 1.80 1.80
02128> | Length (m)= 40.00 10.00
02129> | Mannings n = .013 .025
02130>-----
02131> Max. eff. Inten.(mm/hr)= 232.24 98.18
02132> | over (min) 1.00 2.00
02133> | Storage Coeff. (min)= .88 (ii) 1.68 (iii)
02134> | Unit Hyd. Tpeak (min)= 1.00 2.00
02135> | Unit Hyd. peak (cms)= 1.15 .62
02136>-----
02137> PEAK FLOW (cms)= .01 .05 *TOTALS*
02138> | TIME TO PEAK (hrs)= 1.00 1.02 .059 (iii)
02139> | RUNOFF VOLUME (mm)= 73.83 23.36 25.883
02140> | TOTAL RAINFALL (mm)= 75.83 75.83 75.828
02141> | RUNOFF COEFFICIENT = .97 .31 .341
02142>-----
02143> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02144> CN* = 55.0 Ia = Dep. Storage (Above)
02145> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02146> THAN THE STORAGE COEFFICIENT.
02147> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02148>-----
02149>-----
02150> 001:0110-----
02151>-----
02152> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
02153> | TotalHyd 06:C204 | Number of inlets in system [NINLET] = 1
02154> | Total minor system capacity = .001 (cms)
02155> | Total major system storage [TMJSTO] = 108.(cu.m.)
02156>-----
02157> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02158> | (ha) (cms) (hrs) (mm) (cms)
02159> | TOTAL HYD. 06:C204 .31 .059 1.000 25.883 .000

02160>-----
02161> MAJOR SYST 05:204OVL .00 .000 .000 .000 .000
02162> MINOR SYST 04:INFIL .31 .001 .633 25.893 .000
02163>-----
02164> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02165>-----
02166> Maximum MAJOR SYSTEM storage used = 73.(cu.m.)
02167>-----
02168>-----
02169> 001:0111-----
02170>-----
02171> | CALIB STANDHYD | Area (ha)= 1.13
02172> | 07:C205 DT= 1.00 | Total Imp(%)= 67.00 Dir. Conn.(%)= 35.00
02173>-----
02174> IMPERVIOUS PERVIOUS (i)
02175> | Surface Area (ha)= .76 .37
02176> | Dep. Storage (mm)= 2.00 5.00
02177> | Average Slope (%)= 1.80 1.80
02178> | Length (m)= 40.00 10.00
02179> | Mannings n = .013 .025
02180>-----
02181> Max. eff. Inten.(mm/hr)= 232.24 193.69
02182> | over (min) 1.00 1.00
02183> | Storage Coeff. (min)= .88 (ii) 1.49 (ii)
02184> | Unit Hyd. Tpeak (min)= 1.00 1.00
02185> | Unit Hyd. peak (cms)= 1.15 .83
02186>-----
02187> PEAK FLOW (cms)= .25 .18 *TOTALS*
02188> | TIME TO PEAK (hrs)= 1.00 1.00 1.000
02189> | RUNOFF VOLUME (mm)= 73.83 30.04 45.367
02190> | TOTAL RAINFALL (mm)= 75.83 75.83 75.828
02191> | RUNOFF COEFFICIENT = .97 .40 .598
02192>-----
02193> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02194> CN* = 55.0 Ia = Dep. Storage (Above)
02195> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02196> THAN THE STORAGE COEFFICIENT.
02197> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02198>-----
02199>-----
02200> 001:0112-----
02201>-----
02202> | ADD HYD (to205) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02203> | (ha) (cms) (hrs) (mm) (cms)
02204> | ID1 01:201OVL .29 .014 2.18 11.71 .000
02205> | +ID2 02:202OVL 1.03 .020 1.67 9.23 .000
02206> | +ID3 03:203OVL .00 .000 .00 .00 .000
02207> | +ID4 05:204OVL .00 .000 .00 .00 .000
02208> | +ID5 07:C205 1.13 .436 1.00 45.37 .000
02209> |=====|
02210> | SUM 10:to205 2.45 .436 1.00 26.16 .000
02211>-----
02212> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02213>-----
02214>-----
02215> 001:0113-----
02216>-----
02217> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .023 (cms)
02218> | TotalHyd 10:to205 | Number of inlets in system [NINLET] = 1
02219> | Total minor system capacity = .023 (cms)
02220> | Total major system storage [TMJSTO] = 761.(cu.m.)
02221>-----
02222> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02223> | (ha) (cms) (hrs) (mm) (cms)
02224> | TOTAL HYD. 10:to205 2.45 .436 1.000 26.163 .000
02225>-----
02226> MAJOR SYST 01:205OVL .00 .000 .000 .000 .000
02227> MINOR SYST 02:INFIL 2.45 .023 .683 26.163 .000
02228>-----
02229> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02230>-----
02231> Maximum MAJOR SYSTEM storage used = 414.(cu.m.)
02232>-----
02233>-----
02234> 001:0114-----
02235>-----
02236> | CALIB NASHYD | Area (ha)= .09 Curve Number (CN)=32.00
02237> | 07:EX3 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02238> | U.H. Tp(hrs)= .500
02239>-----
02240> Unit Hyd Qpeak (cms)= .007
02241>-----
02242> PEAK FLOW (cms)= .001 (i)
02243> TIME TO PEAK (hrs)= 1.667
02244> RUNOFF VOLUME (mm)= 8.205
02245> TOTAL RAINFALL (mm)= 75.828
02246> RUNOFF COEFFICIENT = .108
02247>-----
02248> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02249>-----
02250>-----
02251> 001:0115-----
02252>-----
02253> | CALIB NASHYD | Area (ha)= .05 Curve Number (CN)=55.00
02254> | 04:206 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02255> | U.H. Tp(hrs)= .250
02256>-----
02257> Unit Hyd Qpeak (cms)= .008
02258>-----
02259> PEAK FLOW (cms)= .003 (i)
02260> TIME TO PEAK (hrs)= 1.333
02261> RUNOFF VOLUME (mm)= 17.992
02262> TOTAL RAINFALL (mm)= 75.828
02263> RUNOFF COEFFICIENT = .237
02264>-----
02265> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02266>-----
02267>-----
02268> 001:0116-----
02269>-----
02270> | ADD HYD (toDitch) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02271> | (ha) (cms) (hrs) (mm) (cms)
02272> | ID1 04:206 .05 .003 1.33 17.99 .000
02273> | +ID2 07:EX3 .09 .001 1.67 8.20 .000
02274> |=====|
02275> | SUM 10:toDitch .14 .004 1.42 11.70 .000
02276>-----
02277> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02278>-----
02279>-----
02280> 001:0117-----
02281>-----
02282> | CHICAGO STORM | IDF curve parameters: A=3048.220
02283> | Ptotal= 86.60 mm | B= 10.030
02284> | C= .888
02285> | used in: INTENSITY = A / (t + B)^C
02286>-----

02287> Duration of storm = 3.00 hrs
02288> Storm time step = 5.00 min
02289> Time to peak ratio = .33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	
02292>	.08	5.146	.83	43.789	1.58	19.207	2.33	7.218
02294>	.17	5.748	.92	106.492	1.67	16.378	2.42	6.730
02295>	.25	6.505	1.00	274.730	1.75	14.228	2.50	6.302
02296>	.33	7.486	1.08	136.493	1.83	12.548	2.58	5.925
02297>	.42	8.799	1.17	76.628	1.92	11.204	2.67	5.589
02298>	.50	10.637	1.25	50.754	2.00	10.107	2.75	5.289
02299>	.58	13.366	1.33	36.943	2.08	9.197	2.83	5.020
02300>	.67	17.763	1.42	28.579	2.17	8.431	2.92	4.776
02301>	.75	25.782	1.50	23.067	2.25	7.779	3.00	4.554

02302>-----
02303> 001:0118-----
02305>-----
02306> CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
02307> 01:EX1 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02308> U.H. Tp(hrs)= .500
02309> Unit Hyd Qpeak (cms)= .079
02310>-----
02311> PEAK FLOW (cms)= .022 (i)
02312> TIME TO PEAK (hrs)= 1.667
02313> RUNOFF VOLUME (mm)= 10.716
02314> TOTAL RAINFALL (mm)= 86.599
02315> RUNOFF COEFFICIENT = .124
02316>-----
02317> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02318>-----
02319> 001:0119-----
02320>-----
02322> CALIB NASHYD | Area (ha)= 1.04 Curve Number (CN)=32.00
02323> 02:EX2 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02324> U.H. Tp(hrs)= .500
02325> Unit Hyd Qpeak (cms)= .079
02326>-----
02327> PEAK FLOW (cms)= .022 (i)
02328> TIME TO PEAK (hrs)= 1.667
02329> RUNOFF VOLUME (mm)= 10.716
02330> TOTAL RAINFALL (mm)= 86.599
02331> RUNOFF COEFFICIENT = .124
02332>-----
02333> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02334>-----
02335> 001:0120-----
02336>-----
02337> CALIB STANDHYD | Area (ha)= .27
02338> 03:C201 DT= 1.00 | Total Imp(%)= 28.00 Dir. Conn.(%)= 5.00
02339>-----
02340> IMPERVIOUS PERVIOUS (i)
02341> Surface Area (ha)= .08 .19
02342> Dep. Storage (mm)= 2.00 5.00
02343> Average Slope (%)= 1.80 1.80
02344> Length (m)= 40.00 10.00
02345> Mannings n = .013 .025
02346>-----
02347> Max.eff.Inten.(mm/hr)= 274.73 120.91
02348> over (min)= 1.00 2.00
02349> Storage Coeff. (min)= .82 (ii) 1.56 (ii)
02350> Unit Hyd. Tpeak (min)= 1.00 2.00
02351> Unit Hyd. peak (cms)= 1.19 .65
02352>-----
02353> *TOTALS*
02354> PEAK FLOW (cms)= .01 .06 .067 (iii)
02355> TIME TO PEAK (hrs)= 1.00 1.00 1.000
02356> RUNOFF VOLUME (mm)= 84.60 28.54 31.339
02357> TOTAL RAINFALL (mm)= 86.60 86.60 86.599
02358> RUNOFF COEFFICIENT = .98 .33 .362
02359>-----
02360> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02361> CN* = 55.0 Ia = Dep. Storage (Above)
02362> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02363> THAN THE STORAGE COEFFICIENT.
02364> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02365>-----
02366> 001:0121-----
02367>-----
02371> ADD HYD (201trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02372> (ha) (cms) (hrs) (mm) (cms)
02373> ID1 01:EX1 1.04 .022 1.67 10.72 .000
02374> +ID2 03:C201 .27 .067 1.00 31.34 .000
02375>-----
02376> SUM 10:201trench 1.31 .068 1.00 14.97 .000
02377>-----
02378> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02379>-----
02380> 001:0122-----
02381>-----
02382> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
02383> TotalHyd 10:201tre | Number of inlets in system [NINLET] = 1
02384> Total minor system capacity = .001 (cms)
02385> Total major system storage [TMJSTO] = 108.(cu.m.)
02386>-----
02387> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02388> (ha) (cms) (hrs) (mm) (cms)
02389> TOTAL HYD. 10:201tre 1.31 .068 1.00 14.966 .000
02390>-----
02391> MAJOR SYST 01:201OVL .51 .026 1.783 14.966 .000
02392> MINOR SYST 03:INFIL .80 .001 .633 14.967 .000
02393>-----
02394> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02395>-----
02396> Maximum MAJOR SYSTEM storage used = 108.(cu.m.)
02397>-----
02398> 001:0123-----
02399>-----
02400> CALIB NASHYD | Area (ha)= .12 Curve Number (CN)=55.00
02401> 04:202 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02402> U.H. Tp(hrs)= .500
02403> Unit Hyd Qpeak (cms)= .009
02404>-----
02405> PEAK FLOW (cms)= .006 (i)
02406> TIME TO PEAK (hrs)= 1.667
02407> RUNOFF VOLUME (mm)= 22.998
02408> TOTAL RAINFALL (mm)= 86.599
02409> RUNOFF COEFFICIENT = .266
02410>-----

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02411>-----
02412> 001:0124-----
02413>-----
02418> ADD HYD (202trench) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02419> (ha) (cms) (hrs) (mm) (cms)
02420> ID1 02:EX2 1.04 .022 1.67 10.72 .000
02421> +ID2 04:202 .12 .006 1.67 23.00 .000
02422>-----
02423> SUM 09:202trench 1.16 .027 1.67 11.99 .000
02424>-----
02425> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02426>-----
02427> 001:0125-----
02428>-----
02429> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
02430> TotalHyd 09:202tre | Number of inlets in system [NINLET] = 1
02431> Total minor system capacity = .000 (cms)
02432> Total major system storage [TMJSTO] = 7.(cu.m.)
02433>-----
02434> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02435> (ha) (cms) (hrs) (mm) (cms)
02436> TOTAL HYD. 09:202tre 1.16 .027 1.667 11.986 .000
02437>-----
02438> MAJOR SYST 02:202OVL 1.06 .027 1.667 11.986 .000
02439> MINOR SYST 04:INFIL .10 .000 .917 12.040 .000
02440>-----
02441> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02442>-----
02443> Maximum MAJOR SYSTEM storage used = 7.(cu.m.)
02444>-----
02445> 001:0126-----
02446>-----
02447> CALIB NASHYD | Area (ha)= .18 Curve Number (CN)=55.00
02448> 05:203 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02449> U.H. Tp(hrs)= .500
02450>-----
02451> Unit Hyd Qpeak (cms)= .014
02452>-----
02453> PEAK FLOW (cms)= .008 (i)
02454> TIME TO PEAK (hrs)= 1.667
02455> RUNOFF VOLUME (mm)= 23.002
02456> TOTAL RAINFALL (mm)= 86.599
02457> RUNOFF COEFFICIENT = .266
02458>-----
02459> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02460>-----
02461> 001:0127-----
02462>-----
02463> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .000 (cms)
02464> TotalHyd 05:203 | Number of inlets in system [NINLET] = 1
02465> Total minor system capacity = .000 (cms)
02466> Total major system storage [TMJSTO] = 40.(cu.m.)
02467>-----
02468> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02469> (ha) (cms) (hrs) (mm) (cms)
02470> TOTAL HYD. 05:203 .18 .008 1.667 23.002 .000
02471>-----
02472> MAJOR SYST 03:203OVL .00 .000 .000 .000 .000
02473> MINOR SYST 04:INFIL .18 .000 1.000 23.011 .000
02474>-----
02475> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02476>-----
02477> Maximum MAJOR SYSTEM storage used = 37.(cu.m.)
02478>-----
02479> 001:0128-----
02480>-----
02481> CALIB STANDHYD | Area (ha)= .31
02482> 06:C204 DT= 1.00 | Total Imp(%)= 31.00 Dir. Conn.(%)= 5.00
02483>-----
02484> IMPERVIOUS PERVIOUS (i)
02485> Surface Area (ha)= .10 .21
02486> Dep. Storage (mm)= 2.00 5.00
02487> Average Slope (%)= 1.80 1.80
02488> Length (m)= 40.00 10.00
02489> Mannings n = .013 .025
02490>-----
02491> Max.eff.Inten.(mm/hr)= 274.73 130.56
02492> over (min)= 1.00 2.00
02493> Storage Coeff. (min)= .82 (ii) 1.54 (ii)
02494> Unit Hyd. Tpeak (min)= 1.00 2.00
02495> Unit Hyd. peak (cms)= 1.19 .65
02496>-----
02497> *TOTALS*
02498> PEAK FLOW (cms)= .01 .07 .079 (iii)
02499> TIME TO PEAK (hrs)= 1.00 1.00 1.000
02500> RUNOFF VOLUME (mm)= 84.60 29.43 32.187
02501> TOTAL RAINFALL (mm)= 86.60 86.60 86.599
02502> RUNOFF COEFFICIENT = .98 .34 .372
02503>-----
02504> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02505> CN* = 55.0 Ia = Dep. Storage (Above)
02506> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02507> THAN THE STORAGE COEFFICIENT.
02508> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02509>-----
02510> 001:0129-----
02511>-----
02512> COMPUTE DUALHYD | Average inlet capacities [CINLET] = .001 (cms)
02513> TotalHyd 06:C204 | Number of inlets in system [NINLET] = 1
02514> Total minor system capacity = .001 (cms)
02515> Total major system storage [TMJSTO] = 108.(cu.m.)
02516>-----
02517> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02518> (ha) (cms) (hrs) (mm) (cms)
02519> TOTAL HYD. 06:C204 .31 .079 1.000 32.187 .000
02520>-----
02521> MAJOR SYST 05:204OVL .00 .000 .000 .000 .000
02522> MINOR SYST 04:INFIL .31 .001 .600 32.188 .000
02523>-----
02524> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02525>-----
02526> Maximum MAJOR SYSTEM storage used = 92.(cu.m.)
02527>-----
02528> 001:0130-----
02529>-----
02530> CALIB STANDHYD | Area (ha)= 1.13
02531> 07:C205 DT= 1.00 | Total Imp(%)= 67.00 Dir. Conn.(%)= 35.00
02532> U.H. Tp(hrs)= .500
02533>-----
02534> IMPERVIOUS PERVIOUS (i)
02535> Surface Area (ha)= .76 .37

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02541> Dep. Storage (mm)= 2.00 5.00
02542> Average Slope (%)= 1.80 1.80
02543> Length (m)= 40.00 10.00
02544> Mannings n = .013 .025
02545>
02546> Max. eff. Inten. (mm/hr)= 274.73 252.70
02547> over (min) 1.00 1.00
02548> Storage Coeff. (min)= .82 (ii) 1.37 (ii)
02549> Unit Hyd. Tpeak (min)= 1.00 1.00
02550> Unit Hyd. peak (cms)= 1.19 .88
02551>
02552> PEAK FLOW (cms)= .30 .24 *TOTALS*
02553> TIME TO PEAK (hrs)= 1.00 1.00 .541 (iii)
02554> RUNOFF VOLUME (mm)= 84.60 37.28 1.000
02555> TOTAL RAINFALL (mm)= 86.60 86.60 53.839
02556> RUNOFF COEFFICIENT = .98 .43 86.599
02557>
02558> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
02559> CN* = 55.0 Ia = Dep. Storage (Above)
02560> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
02561> THAN THE STORAGE COEFFICIENT.
02562> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02563>
02564> -----
02565> 001:0131-----
02566>
02567> | ADD HYD (to205 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02568> |-----| |-----| |-----| |-----| |-----| |-----|
02569> | ID1 01:2010VLD | | .51 .026 1.78 14.97 .000
02570> | +ID2 02:2020VLD | | 1.06 .027 1.67 11.99 .000
02571> | +ID3 03:2030VLD | | .00 .000 .00 .00 .000
02572> | +ID4 05:2040VLD | | .00 .000 .00 .00 .000
02573> | +ID5 07:C205 | | 1.13 .541 1.00 53.84 .000
02574> |-----| |-----| |-----| |-----| |-----| |-----|
02575> | SUM 10:to205 | | 2.70 .541 1.00 30.05 .000
02576>
02577> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02578>
02579> -----
02580> 001:0132-----
02581>
02582> | COMPUTE DUALHYD | Average inlet capacities [CINLET] = .023 (cms)
02583> | TotalHyd 10:to205 | Number of inlets in system [NINLET] = 1
02584> |-----| |-----| |-----| |-----| |-----| |-----|
02585> | | Total minor system capacity = .023 (cms)
02586> | | Total major system storage [TMJSTO] = 761.(cu.m.)
02587>
02588> ID: NHYD AREA QPEAK TPEAK R.V. DWF
02589> (ha) (cms) (hrs) (mm) (cms)
02590> TOTAL HYD. 10:to205 2.70 .541 1.000 30.053 .000
02591> =====
02592> MAJOR SYST 01:2050VL .00 .000 .000 .000 .000
02593> MINOR SYST 02:INFIL 2.70 .023 .683 30.072 .000
02594>
02595> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02596>
02597> Maximum MAJOR SYSTEM storage used = 576.(cu.m.)
02598>
02599> -----
02600> 001:0133-----
02601> | CALLB NASHYD | Area (ha)= .09 Curve Number (CN)=32.00
02602> | 07:EX3 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02603> |-----| |-----| |-----| |-----| |-----| |-----|
02604> | | U.H. Tp(hrs)= .500
02605>
02606> Unit Hyd Qpeak (cms)= .007
02607>
02608> PEAK FLOW (cms)= .002 (i)
02609> TIME TO PEAK (hrs)= 1.667
02610> RUNOFF VOLUME (mm)= 10.706
02611> TOTAL RAINFALL (mm)= 86.599
02612> RUNOFF COEFFICIENT = .124
02613>
02614> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02615>
02616> -----
02617> 001:0134-----
02618> | CALLB NASHYD | Area (ha)= .05 Curve Number (CN)=55.00
02619> | 04:206 DT= 5.00 | Ia (mm)= 5.000 # of Linear Res.(N)= 3.00
02620> |-----| |-----| |-----| |-----| |-----| |-----|
02621> | | U.H. Tp(hrs)= .250
02622>
02623> Unit Hyd Qpeak (cms)= .008
02624>
02625> PEAK FLOW (cms)= .004 (i)
02626> TIME TO PEAK (hrs)= 1.333
02627> RUNOFF VOLUME (mm)= 22.993
02628> TOTAL RAINFALL (mm)= 86.599
02629> RUNOFF COEFFICIENT = .266
02630>
02631> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
02632>
02633> -----
02634> 001:0135-----
02635> | ADD HYD (toDitch ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
02636> |-----| |-----| |-----| |-----| |-----| |-----|
02637> | ID1 04:206 | | .05 .004 1.33 22.99 .000
02638> | +ID2 07:EX3 | | .09 .002 1.67 10.71 .000
02639> |-----| |-----| |-----| |-----| |-----| |-----|
02640> | SUM 10:toDitch | | .14 .005 1.33 15.09 .000
02641>
02642> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
02643>
02644> -----
02645> 001:0136-----
02646> FINISH
02647>
02648> *****
02649> WARNINGS / ERRORS / NOTES
02650>
02651> Simulation ended on 2021-07-14 at 12:15:15
02652> *****
02653>
02654>

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